WSPG Documentation

Table of Contents

WSPG Introduction 1
Forward 1
Introduction to WSPG 1
Purpose of WSPG 1
Distributed Versions of WSPG 1
General Program Description 3
Introduction 3
Basic Theory 3
Computational Procedures 3
Flow Rates 3
Manning's "n" 3
Starting Water Surface Elevations 3
Critical and Normal Depths 4
Velocity Head 4
Water Surface Stages 4
Data Processing Description 5
Data Processing System Description 5
Element Description 7
Element Types and Construction 7
Boundary Lines 7
System Outlet (SO) 8
System Headworks (SH) 9
Reach (R) 10
Junction Structure (JX) 11
Join Structure (JO) 12
Transition Structure (TS) 12
Bridge Entrance (BE) 13
Bridge Exit (BX) 14
Wall Entrance (WE) 15
Wall Exit (WX) 16
Channel and Conduit Section Description 19
Channel and Conduit Section Description 19
Regular Channel Type Sections 19
Irregular Sections 20
Definitions & Restrictions for Irregular Sections 21
Data Input Descriptions 23
Data Input Descriptions 23
WSPG Job Control 23
Link Element Data Dialog 23
Reach 24
Transition 25
Node Element Data 26
System Headworks 27
System Outlet 28
Wall Entrance 29
Wall Exit 30
Bridge Entrance 31

Bridge Exit 32
Junction 33
Lateral Branches 34
Join 35
Channel Selection 36
Channel Types 37
Trapezoidal Open 37
Rectangular Open 38
Trapezoidal / Rectangular Closed 39
Pipe 40
Irregular 41
Irregular channel editor 42
Pier Elevations Input 43
Warning Messages 45
Warning messages 45
Error Messages 49
Error Messages 49
Index 53

WSPG Introduction

Forward

The use of this program by others is made and accepted with the understanding the XP Software and Los Angeles County Public Works Department makes no warranties, express or implied, concerning its accuracy, completeness, reliability, usability or suitability and the District shall be under no liability for any use made thereof.

Introduction to WSPG

Water Surface Pressure Gradient (WSPG) is a hydraulic analysis model that computes and plots uniform and non-uniform steady flow water surface profiles and pressure gradients in open channels or closed conduits with irregular or regular sections. The Los Angeles County Department of Public Works (LADPW) requires use of this model for hydraulic design.

This program was originally developed by the Design Systems and Standards Group of the Design Division and the Data Processing Section of the Business and Fiscal Division of the Los Angeles County Flood Control District. This program was originally written as a mainframe program called F0515P for use by the Los Angeles County Flood Control District or by its Contractors on District projects. The program was written in FORTRAN IV, compiled using the IBM FORTRAN H compiler executing on an IBM 370/158 using OS/VS2 MVS. The system required the use of an input media (such as a card reader), temporary disk storage, and a printer. It was designed to run in batch mode.

In the past there have been two implementations of the WSPG: a public domain DOS program, and a menu-driven, non-graphical Windows interface.

In April 2009, the LADPW contracted with XP Software to update the WSPG hydraulic engine and embed that engine in a graphical user interface, namely the XP Software interface for xpswmm/xpstorm.

Purpose of WSPG

The program computes and plots uniform and non-uniform steady flow water surface profiles and pressure gradients in open channels or closed conduits with irregular or regular sections. The flow in a system may alternate between super critical, subcritical or pressure flow in any sequence. The program will also analyze natural river channels although the principle use of the program is intended for determining profiles in improved flood control systems.

The collaborative effort between XP Software and the LADPW has resulted in WSPG being available in three forms:

Distributed Versions of WSPG

wspg2010: Updated code and addition of the JOIN element, including a simple graphical interface. This version is provided free of charge by XP Software and is available for download from www.xpsoftware.com.

xpwspg2010: A standalone product that includes XP Software's powerful geospatial working environment with WSPG hydraulic calculations, profile views, export to CAD, extract network data from Digital Terrain Models, easy data entry with xptables and much more. This product also provides access to the Modified Rational method (MODRAT) and an automated facility to obtain hydrologic peak flows for upstream headworks nodes for simulation in the WSPG hydraulic calculations. **xpwspg2010** is available for purchase through XP Software.

xpwspg add-on module: An add-on module for existing xpswmm and xpstorm users. The add-on module adds the WSPG hydraulic calculations to the comprehensive XP packages. The add-on module couples the LADPW hydrologic methods with the many existing features such as the dynamic St. Venant calculations, GIS data exchange, reports, and water quality components available in XP Software.

General Program Description

Introduction

XP Software reviewed and updated the original LADPW code for WSPG. Notable modifications are herein indicated. The XP Software re-write of WSPG primarily converted the FORTRAN to the more current programming code, C++. The capability to handle a branching (dendritic) network was added.

Basic Theory

The computational procedure is based on solving Bernoulli's equation for the total energy at each section and Manning's formula for friction loss between the sections in a reach. The open channel flow procedure utilizes the standard step method. Confluences and bridge piers are analyzed using pressure and momentum theory.

The program uses basic mathematical and hydraulic principles to calculate all such data as cross sectional area, wetted perimeter, normal depth, critical depth, pressure, and momentum.

The program processes the computations in three phases: Analysis of the system in the downstream direction (Phase I), analysis of the system in the upstream direction (Phase II), and analysis of the downstream profile (from Phase I) and the upstream profile (from Phase II) to obtain a composite profile (Phase III). The processing was designed to continue calculating unless gross errors are encountered. Warning messages may be issued concerning tolerance levels not being reached on an iterative approximation. These may or may not affect the overall solution to the problem; however, processing continues. If gross errors are encountered, an error message will be issued and processing will stop.

Computational Procedures

Input Preparation

The channel or conduit system is initially subdivided into the following elements: system outlet, reach, transition, confluence (junction), bridge exit, bridge entrance, wall entrance (sudden contraction), wall exit (sudden expansion), and system headworks. Each element is internally assigned a number. Additional "intermediate points" between elements are internally set by the program. Stationing is entered by the user only at the System Outlet (S.O.); the program uses the "length of element" to calculate the stationing.

The entire input is thoroughly scanned for required information and range values of optional information before processing begins. If any errors are detected, processing will stop. Warnings may be issued, but the program will not be prevented. The input data must consist of a minimum of three elements (system outlet, system headwork and other element). The original WSPG program was limited to a maximum of 200 elements. With the XP Software update to WSPG, that limitation was removed.

Flow Rates

The starting flow rate (Q) at the upstream terminus of a system is specified on a "Q" card. The flow rate (Q) is increased at the desired locations by specifying lateral inflow rates on the "JX" cards. The flow rate can be reduced by using a negative lateral "Q".

Manning's "n"

The program uses the Manning formula for the friction loss in all types of conduits or natural channels. The program can only take one "n" value per element; however, the "n" value can change at subsequent elements. If a section has a lining composed of different roughness coefficients a composite "n" based on anticipated depth of flow should be hand computed. If an "n" value is not specified with the input data, the program uses a value of .013.

Starting Water Surface Elevations

Starting water surface elevation at the downstream terminus (System Outlet S.O.) or the upstream terminus (System Headworks S.H.) are optional input values. If not specified, the program will use critical depth elevation to begin computations.

Critical and Normal Depths

Critical depth is computed for every section for the given Q utilizing the "Specific Energy Equation". Normal depth is computed in every reach element on a positive slope for the specified Q.

Velocity Head

The velocity head (HV) is computed using the mean velocity of the section. This may not be accurate in the case of a complex section such as one with shallow flow in the horizontal overbank area where velocity distribution is not uniform. If the program is to be used in this situation the user should be aware that some error may be introduced in the results.

Water Surface Stages

As previously stated, the program processes in three stages. The lower stage water surface (w.s.) profile begins at the system headworks and ends at the system outlet. The computation will proceed downstream in every consecutive element as long as energy is available to maintain flow in the supercritical stage. When energy becomes expended at any point in an element, the lower stage profile will be discontinued from that point to the downstream end of that element. Then computation will resume in the next element with a critical depth control until the system outlet is analyzed.

The upper stage w.s. profile begins at the system outlet and ends at the headworks. Computation proceeds upstream in every element as long as the water surface at the downstream end of any two adjacent points can support the moving mass of water to flow at the critical or subcritical depth. Otherwise, computation will be discontinued from the downstream point to the upstream end of that element. Then computation will resume at the downstream end of the next element with critical depth control, provided no depth less than critical depth has been computed at that point on the lower stage profile. Then computation will proceed up stream until the system headworks is analyzed.

If the computed depth of flow in any open section exceeds the given section height the program will extend vertical walls; a note is provided in the output file indicating the computed water surface elevation is greater than the ground elevation.

The jump routine begins at the system outlet and ends at the headworks. It searches the lower stage and the upper stage profiles for points of equal energy. If a jump is encountered, it will be approximately located, and data on either the upper stage or lower stage not consistent with the greater energy theory will be deleted from every element. The final profile will be a composite of upper stage and lower stage with hydraulic jumps in between.

Data Processing Description

Data Processing System Description

Prior to the XP Software update to the WSPG program, the program was written in FORTRAN IV code, compiled using the IBM FORTRAN H compiler executing on an IBM 370/158 using OS/VS2 MVS. This system required the use of an input media (such as a card reader), temporary disk storage, and a printer. It is designed to run in batch mode. The XP Software update, intended for use on personal computers, used C++ code, includes a graphical user interface and added the Join element, allowing branching (dendritic) networks to be included in a single model.

Data required to build a stream network model is similar to the DOS version of WSPG. Required data is now input through a series of dialogs.

The entire input is thoroughly scanned for required information and range values of optional information before processing begins. If any errors are detected, processing will stop. Warnings may be issued, but they will not prevent processing.

Processing consists of three phases: Analysis of the system in the downstream direction (Phase I), analysis of the system in the upstream direction (Phase II), and analysis of the downstream profile (from Phase I) and the upstream profile (from Phase II) to obtain a composite profile (Phase III). The processing was designed to continue calculating unless gross errors are encountered. Warning messages may be issued concerning tolerance levels not being reached on an iterative approximation. These may or may not affect the overall solution to the problem, however, processing continues. If gross errors are encountered, an error message will be issued and processing will stop.

Output of the system consists of an output file that can be viewed from within the program or externally using a text editor.

Element Description

Element Types and Construction

The channel, conduit or natural river system to be analyzed is subdivided into elements. The XP Software version of WSPG represents a change in the order and manner in which a model is constructed. The geographical interface maintains geographical orientation (x,y) of elements. To maintain flow direction, the drainage network is digitized from upstream to downstream.

The element types are:

Boundary Lines

System Outlet

System Headworks

Reach

Junction Structure

Join Structure

Transition Structure

Bridge Entrance

Bridge Exit

Wall Entrance

Wall Exit

Boundary Lines

All elements are bounded on the upstream end by Section 1 and the downstream end by Section 2 except System Outlet (SO) and System Headworks (SH) which only have Section 1. The user inputs data such as base width, conduit height, etc. for Section 1 of every element. The data for Section 2 for every element is taken by the program from the upstream Section 1 of the adjacent downstream element. Elements may have considerable length between Section 1 and Section 2 as in a reach element or may have a zero length as in a bridge entrance element.

L = length of element

X= number of the element under consideration

X+1= adjacent upstream element

X-1= adjacent downstream element

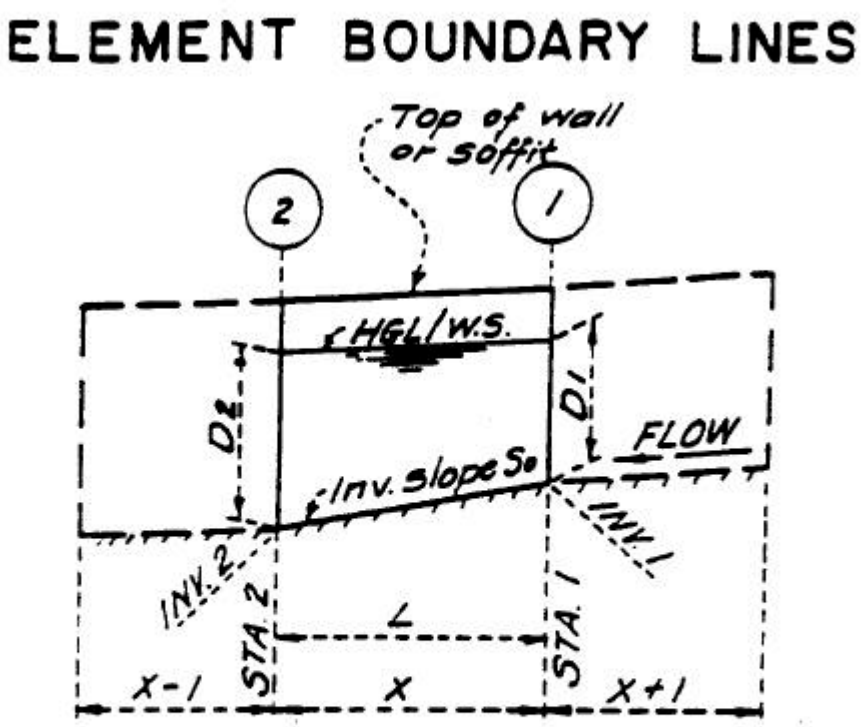


FIG. 5 - 1

System Outlet (SO)

The system outlet is the downstream terminus of a channel. X is equal to one. X+1 can be any element except a System Headworks (SH).

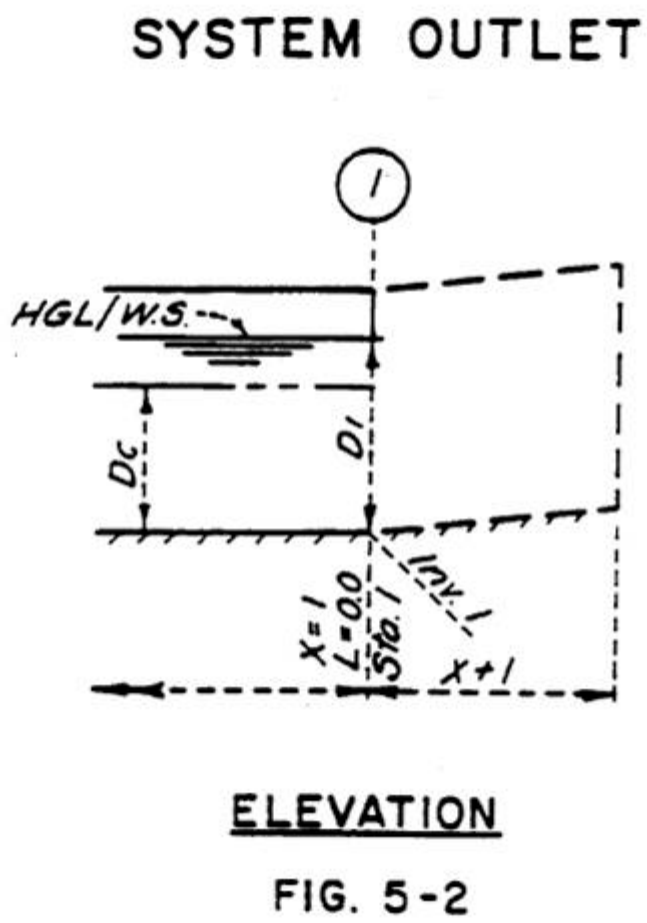
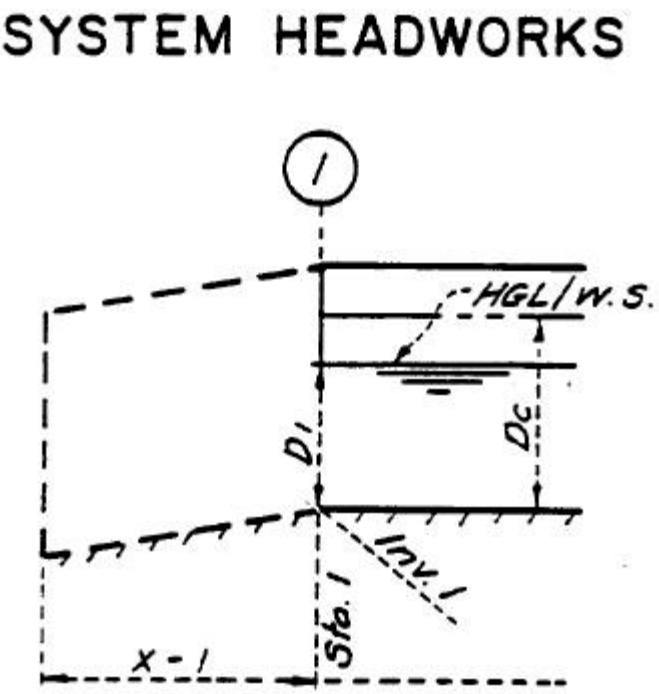


FIG. 5-2

Dialog Box: System Outlet

System Headworks (SH)

The system headworks is the upstream terminus of a channel. Element X-1 can be any element except a system outlet.



Dialog Box: System Headworks

Reach (R)

The reach element is a length of channel, drain or natural river with a constant invert slope, Q, cross section and Manning's n. A reach may have a straight or curving horizontal alignment, however, a curved reach must coincide with the beginning and end of the curve. The same applies to an angle point in the horizontal alignment; a reach must end or begin at the angle point.

Channel types may be either opened or closed, regular or irregular. In open channels (regular rectangular or trapezoidal sections) the super elevation of the water surface is computed and printed for each point in the curve. In pressure flow, bend losses, angle point losses, and manhole losses are computed and add to the friction loss for the reach. Element X+1 can be any element except a system outlet. Element X-1 can be any element except a system headworks.

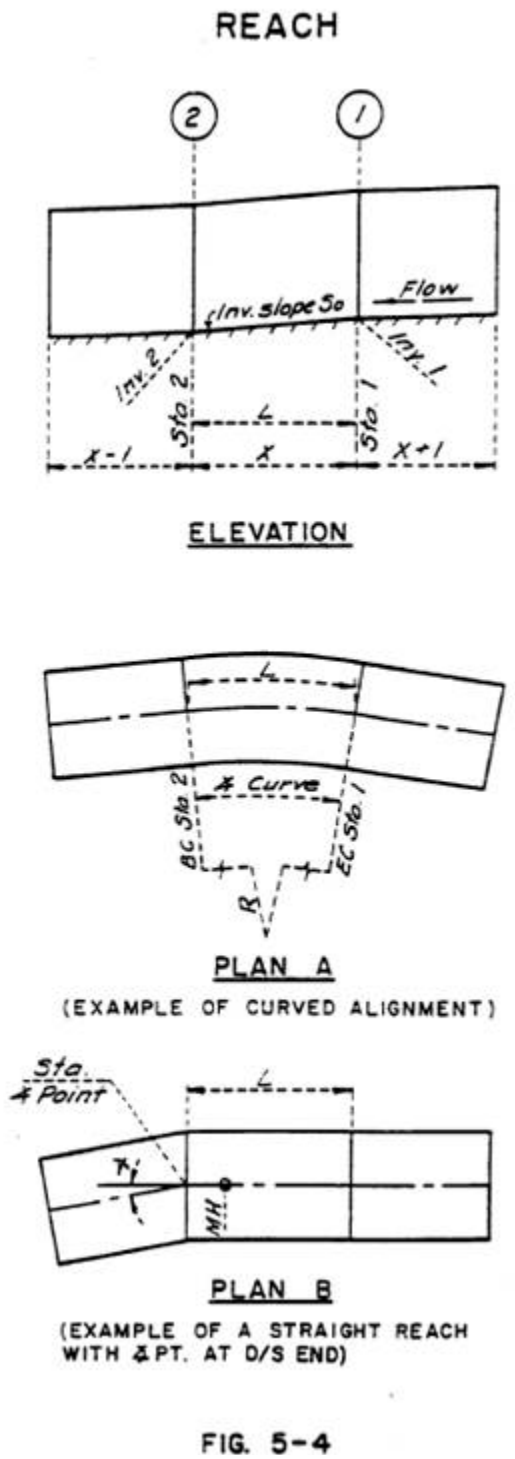


FIG. 5-4

Dialog Box: Reach

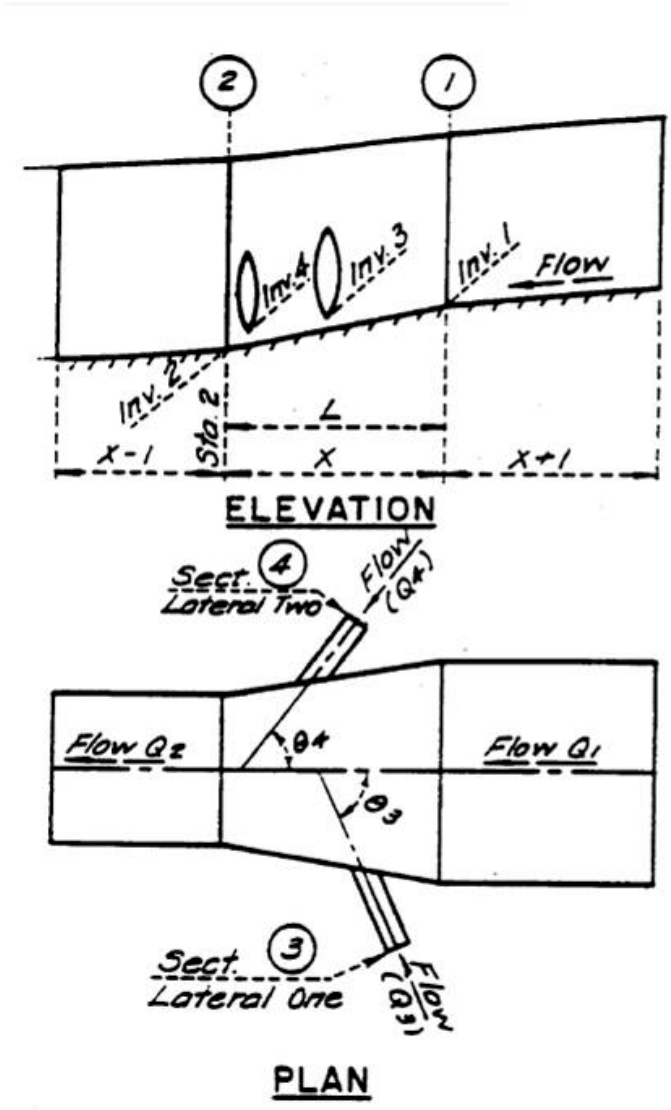
Junction Structure (JX)

The junction structure element is used where there is lateral inflow into the system. Two different laterals can be handled by this element. Element X-1 can be any other element except a System Headworks (SH). Element X+1 can be any other element except a System Outlet (SO).

Dialog Box: Junction

Join Structure (JO)

The join structure element is used where there is lateral inflow into the system. Element X-1 can be any other element except a System Headworks (SH). Element X+1 can be any other element except a System Outlet (SO).



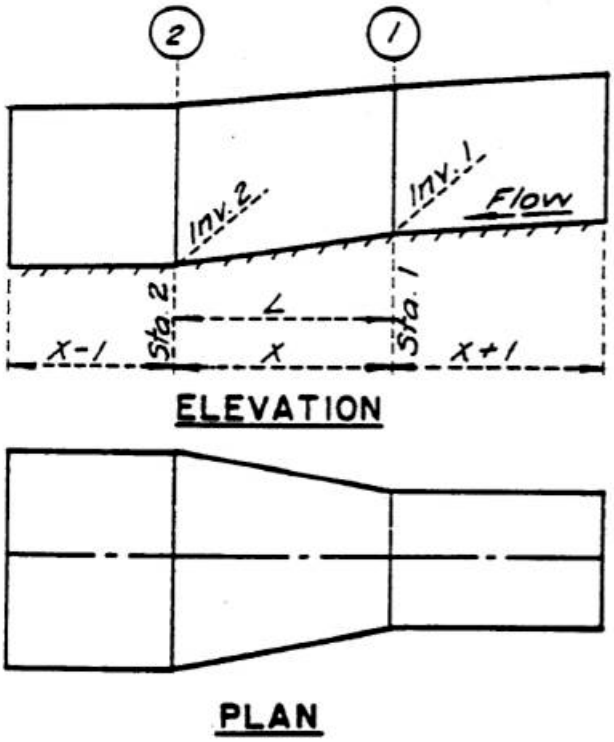
JUNCTION

FIG. 5-5

Dialog Box: Join

Transition Structure (TS)

A transition structure is a gradual expansion or contraction from Section 1 to Section 2. The length L may be any positive number. Element X+1 may be any element except a system outlet (SO). Element X-1 may be any element except a system headworks (SH).



TRANSITION

FIG. 5-6

Dialog Box: Transition

Bridge Entrance (BE)

A bridge entrance is an element used where flow enters from an element without piers into an element with piers.

A bridge entrance is considered to have a zero length element even though the bridge pier nose may have a minor length.

The user should supply the pier area reduction factor. Value must be between 0 and 1. The program uses a default value of 1.0 (See Hydraulic Handbooks for typical values).

Element X-1 may be a SO, R, JX, JO or TS.

Element X+1 may be an R, JX, JO, TS or SH.

It is noted that neither Section 1 nor 2 can be a pipe.

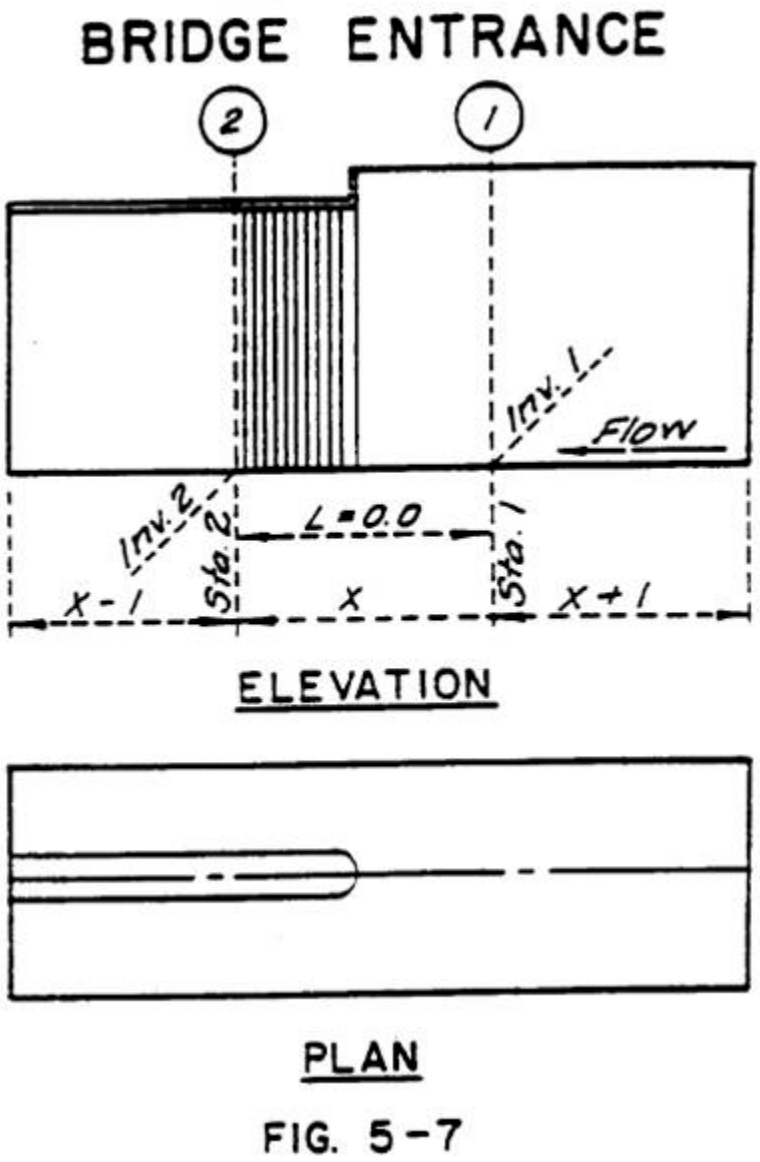


FIG. 5-7

Dialog Box: Bridge Entrance

Bridge Exit (BX)

The bridge exit is also considered to have a zero length element.

A bridge exit is an element used where flow exits from an element with piers into an element without piers.

Element X-1 may be a SO, R, JX, JO or TS.
Element X+1 may be a SH, R, JX, JO or TS.
It is noted that neither Section 1 nor 2 can be a pipe.

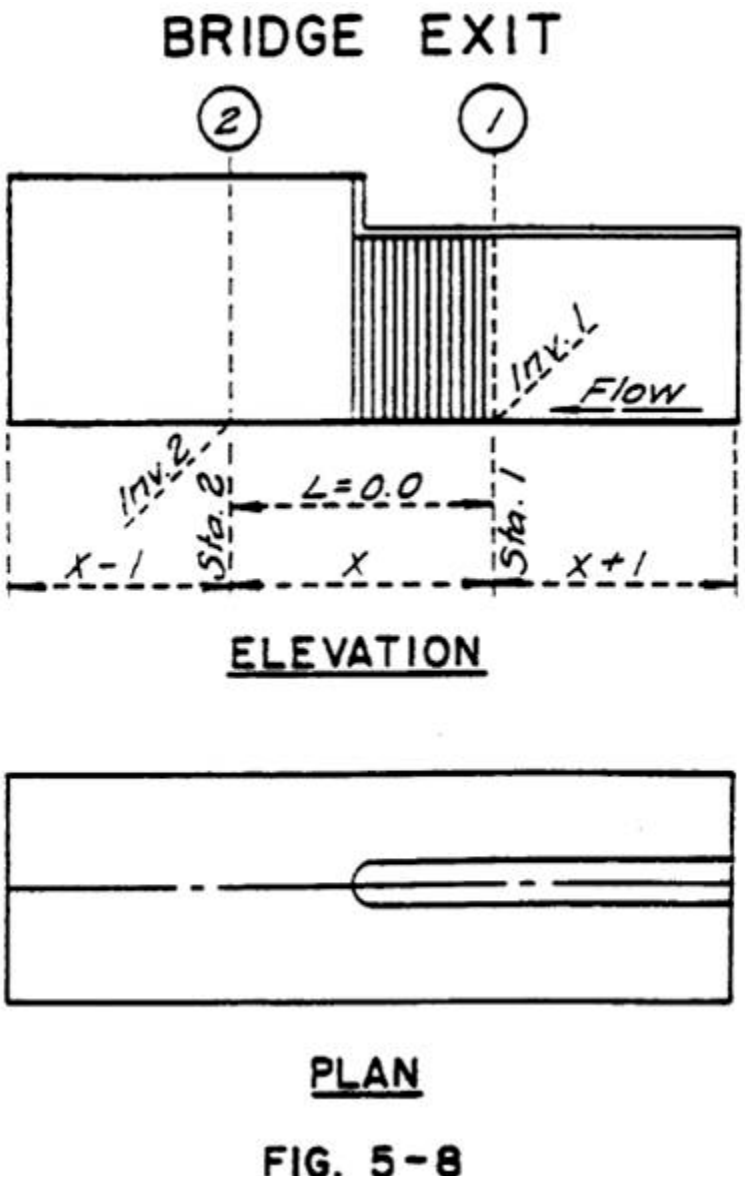


FIG. 5-8

Dialog Box: Bridge Exit

Wall Entrance (WE)

This element is used when there is a sudden change in the conduit section such as a headwall or an abrupt contraction. This element is considered to have a zero length.

The user should supply the loss coefficient kc expressed in terms of the velocity head. The program uses a default value of 0.5 for kc. (See Hydraulic Handbooks for typical values.)

Element X-1 may be a SO, R, JX, JO or TS.

Element X+1 may be a SH, R, JX, JO or TX.

The section for element X+1 cannot have piers, however, it can be an open channel or closed conduit. The section for element X-1 can also be an open channel or closed conduit and it can be with or without piers.

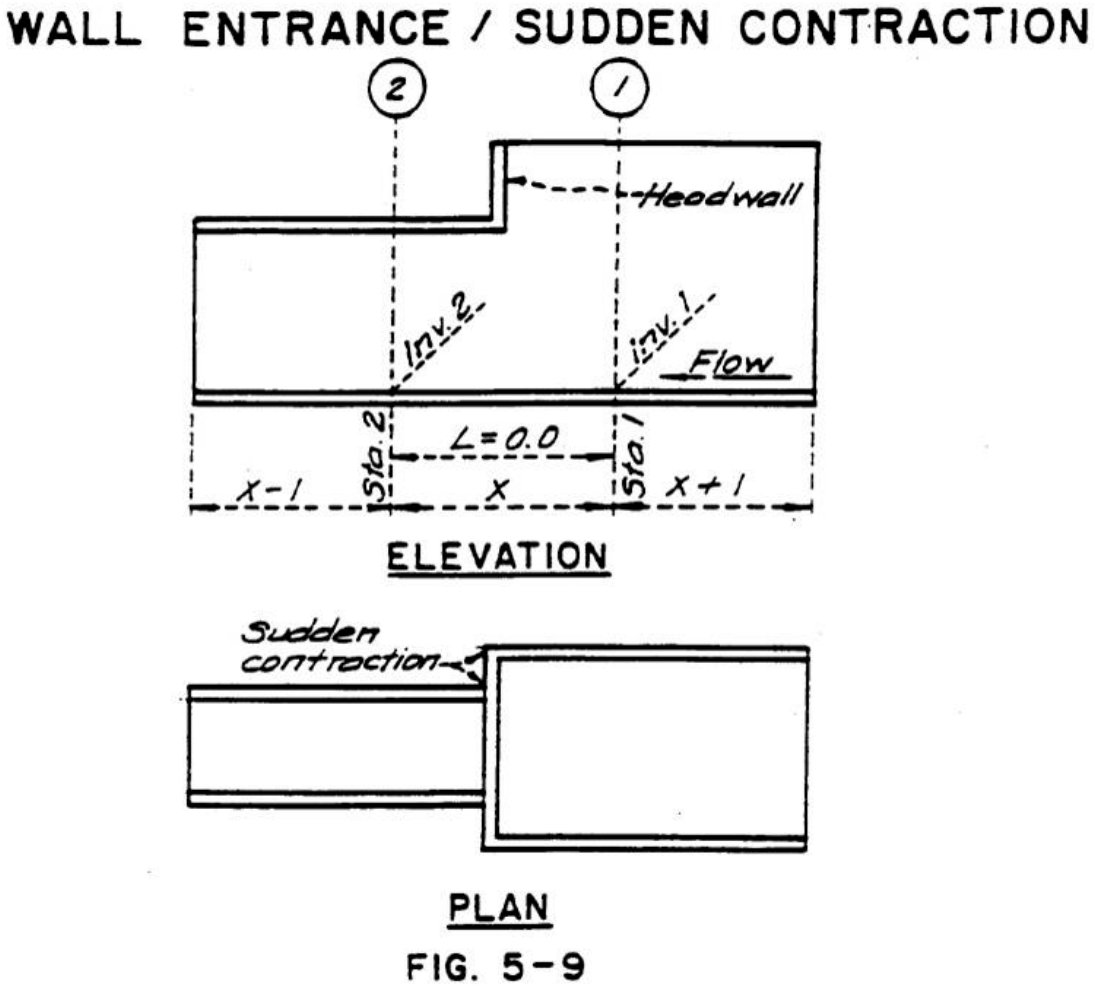


FIG. 5-9

Dialog Box: Wall Entrance

Wall Exit (WX)

This element is used when there is a sudden expansion from a smaller to a larger channel or conduit section. This element is considered to have a zero length.

Element X-1 may be a SO, R, JX, JO or TS.

Element X+1 may be a SH, R, JX, JO or TS.

The section for element X+1 may be an open channel or closed conduit with or without piers. The section for element X-1 may be an open channel or closed conduit, however, it cannot have piers.

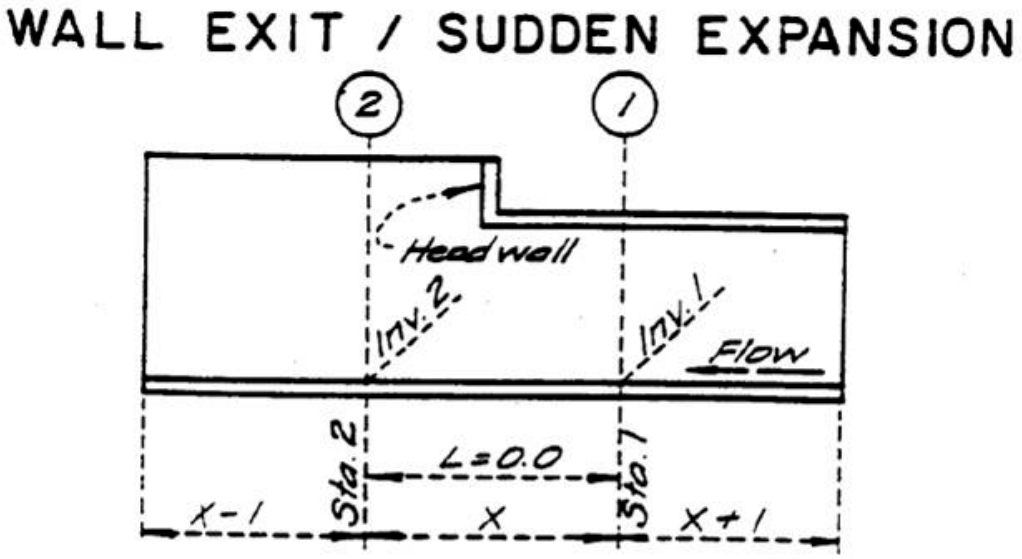


FIG. 5-10

Dialog Box: Wall Exit

Channel and Conduit Section Description

Channel and Conduit Section Description

Channels and conduits sections are classified as regular or irregular sections. The regular sections (Channel Types #1 - #4) are trapezoidal, rectangular channels, box conduits or pipes. The irregular sections (Channel Types #5 and #6) can be natural river sections or irregular shaped improved sections with or without a cover. Piers or center walls can be included in any section except a pipe section.

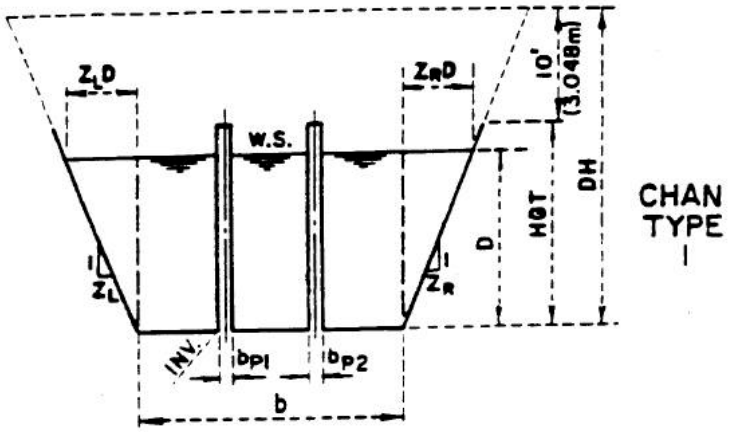
Regular Channel Type Sections

The program utilizes the following regular sections:

Chan. Type #1: Trapezoidal open top with or without piers.

FIG. 6-1

Trapezoidal section with or without piers.

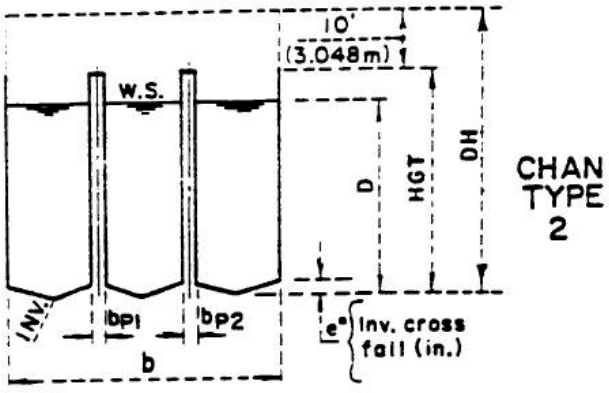


Dialog Box: Trapezoidal Open

Chan. Type #2: Rectangular open top with or without piers.

FIG. 6-2

Rectangular section with or without piers.

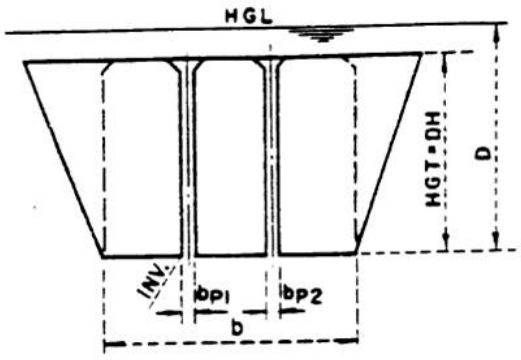


Dialog Box: Rectangular Open

Chan. Type #3: Box, covered trapezoidal or covered rectangular with or without piers.

FIG. 6-3

Box culvert, covered trap or rectangular section with or without piers.



CHAN TYPE 3

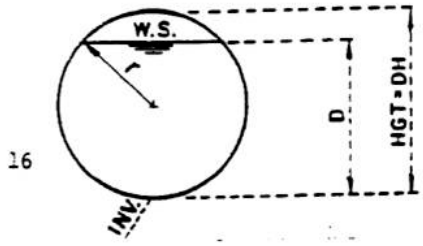
Dialog Box: Trapezoidal / Rectangular Closed

Chan. Type #4: Circular “pipe” one cell only.

FIG. 6-4

Circular section (pipe) one cell only.

9-28-81



CHAN TYPE 4

Dialog Box: Pipe

Note: in multiple cell sections the cells may have variable width but must be of equal height and on the same invert elevation. The top elevations of all piers in regular or irregular channels are assumed equal.

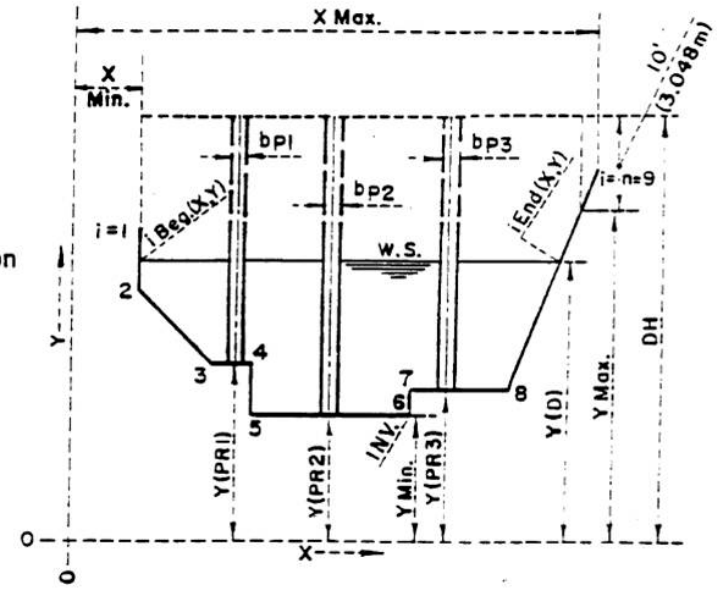
Irregular Sections

The program utilizes the following irregular cross sections:

Chan. Type #5: Irregular open top with or without piers.

FIG. 6-5

Irregular open top section with or without piers.



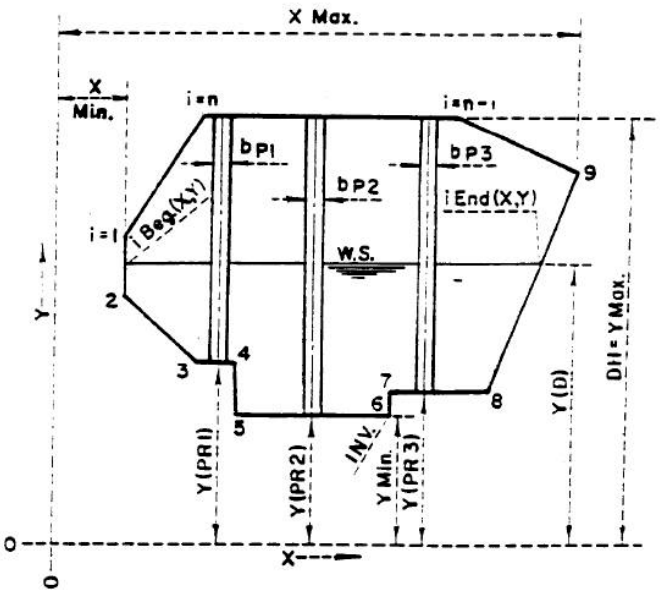
CHAN TYPE 5

Dialog Box: Irregular

Chan. Type #6: Irregular covered with or without piers.

FIG. 6-6

Irregular covered section with or without piers.



CHAN TYPE 6

Dialog Box: Irregular

Definitions & Restrictions for Irregular Sections

An irregular cross section (facing upstream) is defined by x and y coordinates of points I (x,y) given in a counter clockwise direction, from point i=1 to point i=n (minimum 3 points).

Point i = 1 (x,y) is where

x (1) = x min and if x(2) is also x minimum then

y (1) is greater than y (2)

Limitations

Flow Line

Section Shape

Piers

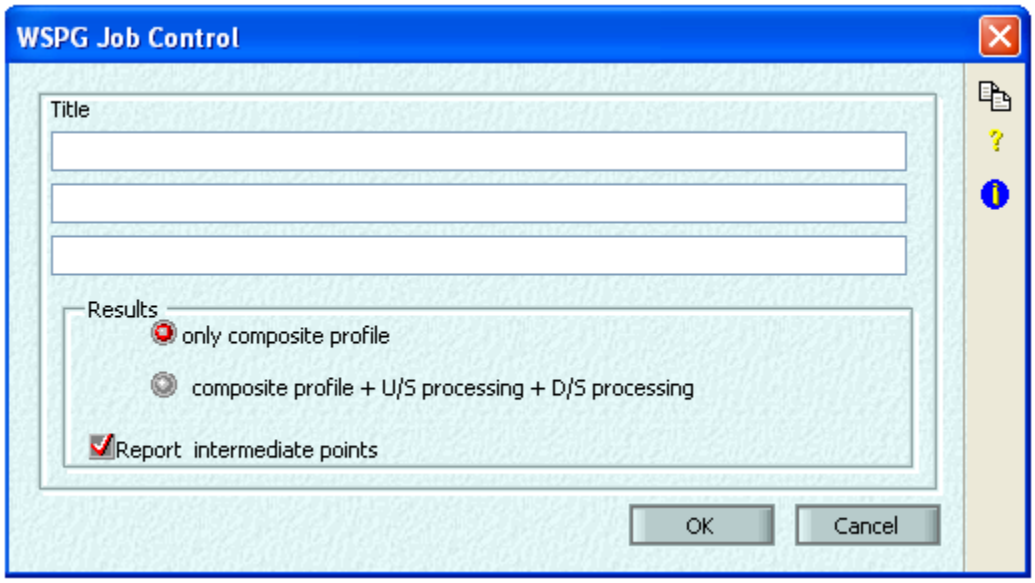
Location of X and Y axis:

Data Input Descriptions

Data Input Descriptions

The user creates link-type elements, reaches, transitions, junctions and joins. Once the link is created, starting at the upstream end of the stream, nodes are automatically inserted at each end of a link. These nodes are set to default to dummy nodes. A dummy node has no effect on the computations. The user can then assign a node-type to these nodes or leave as a dummy node. Each node-type element has one incoming link and one outgoing link, with exception of the System Outlet, System Headwork and a Join. All units are US Customary.

WSPG Job Control

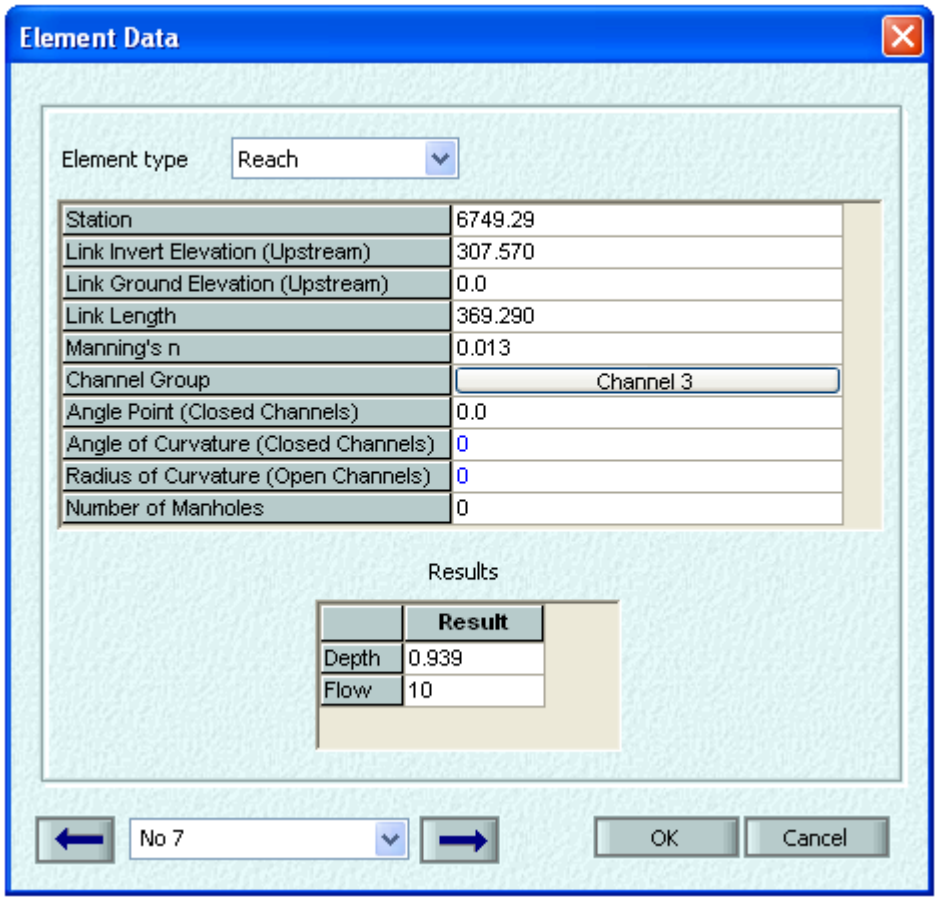


Title allows 3 lines of file description and title information.

The Results radio button indicates the output data presented in the output file created by solving the model.

Reporting of intermediate points in the output file can be toggled on or off.

Link Element Data Dialog



The Element Data dialog allows selection of element type from a pull down list and allows the user to input data associated with that element. Data input requirements vary based on the element selected. For reference, a quick view of the results for the selected element is displayed in the Element Data dialog, after the WSPG model has been solved.

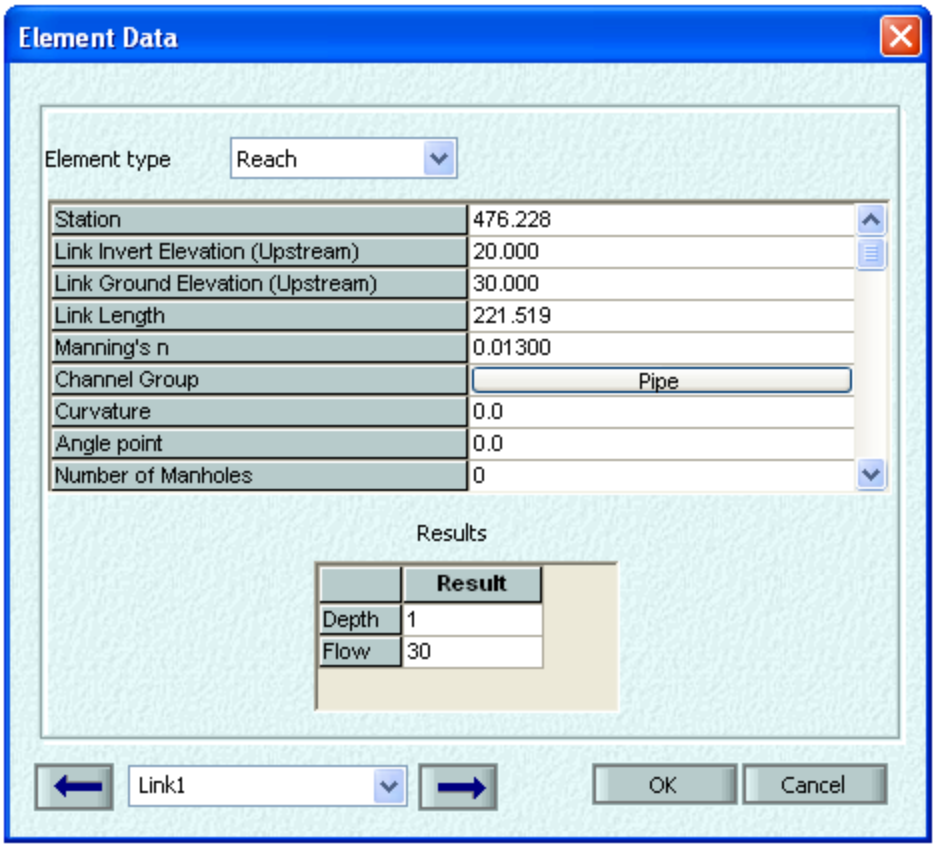
The arrows progress the selection either upstream or downstream in the stream network.

Station data can only be input at the System Outlet. The program automatically calculates stations for the remainder of the system based on the length of each subsequent element.

Element data represent the upstream end of the element. Node Invert Elevation is the elevation of the element at the upstream end of the element. Node Ground Elevation is the elevation at which the element is assumed to no longer contain the flow.

Channel Group is used to select the channel type (types 1-6).

Reach



The Reach element is a length of channel, drain, or natural river with a constant invert slope, flow rate, cross section and Manning's n-value. Element data for each Reach must be entered in the Element Data dialog. Link length must be entered in feet. Manning's "n" values must be entered. The program default is n=0.013.

Angle Point for closed channels is entered in degrees. If an angle point is used it is assumed to occur at the upstream end of the reach element. The maximum recommended angle point is 15 degrees.

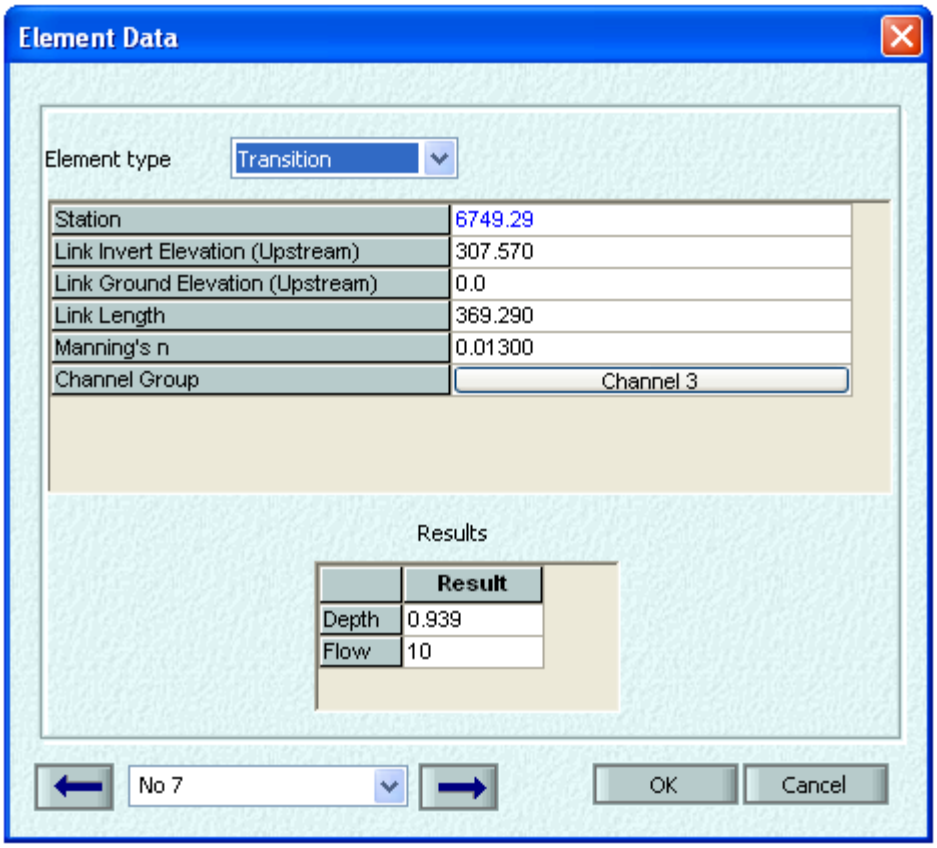
Angle of Curvature for closed channels is entered in degrees. If an angle of curvature is input to represent a curve in the horizontal alignment of the reach, it is assumed for the entire length of the reach, from start to end.

Radius of Curvature for open channels is entered in feet and has similar restrictions as the Angle of Curvature.

Direction of curvature based on looking upstream in the system, positive (+) angles turn right and negative (-) angles turn left.

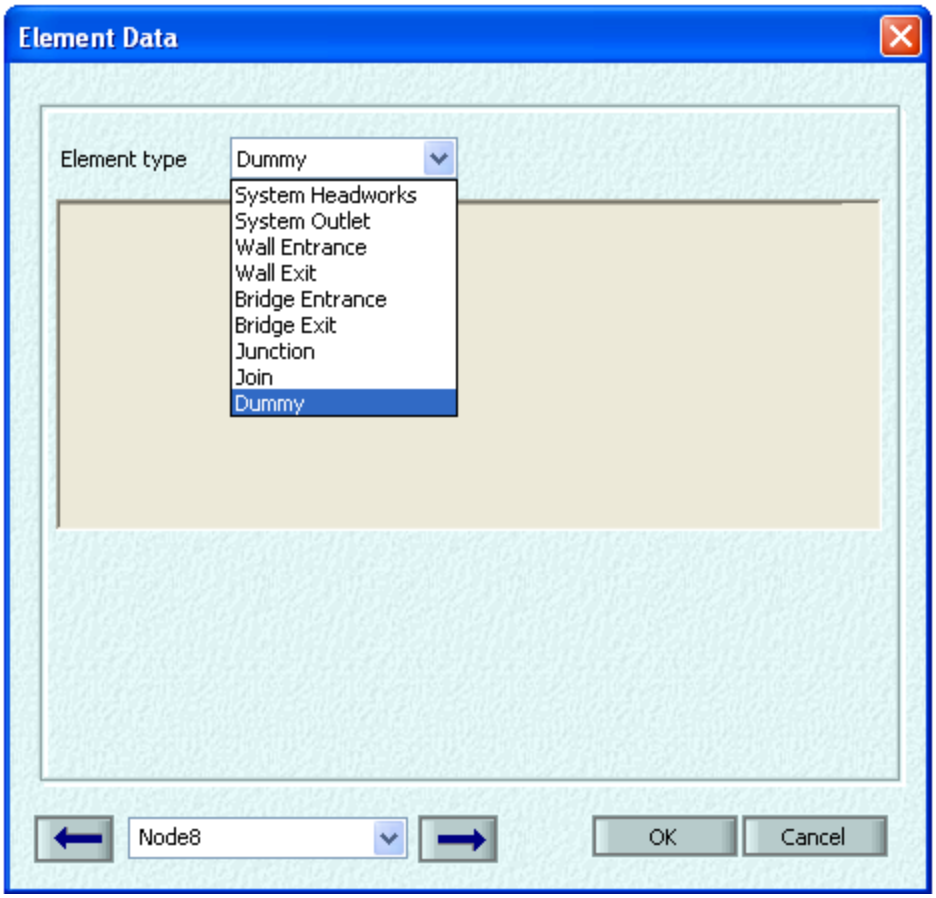
Number of manholes is used for the manhole loss calculations.

Transition



A transition is a link element for which the channel type is changing from the upstream to the downstream end. The upstream section is input, the downstream section is retrieved from the downstream element.

Node Element Data

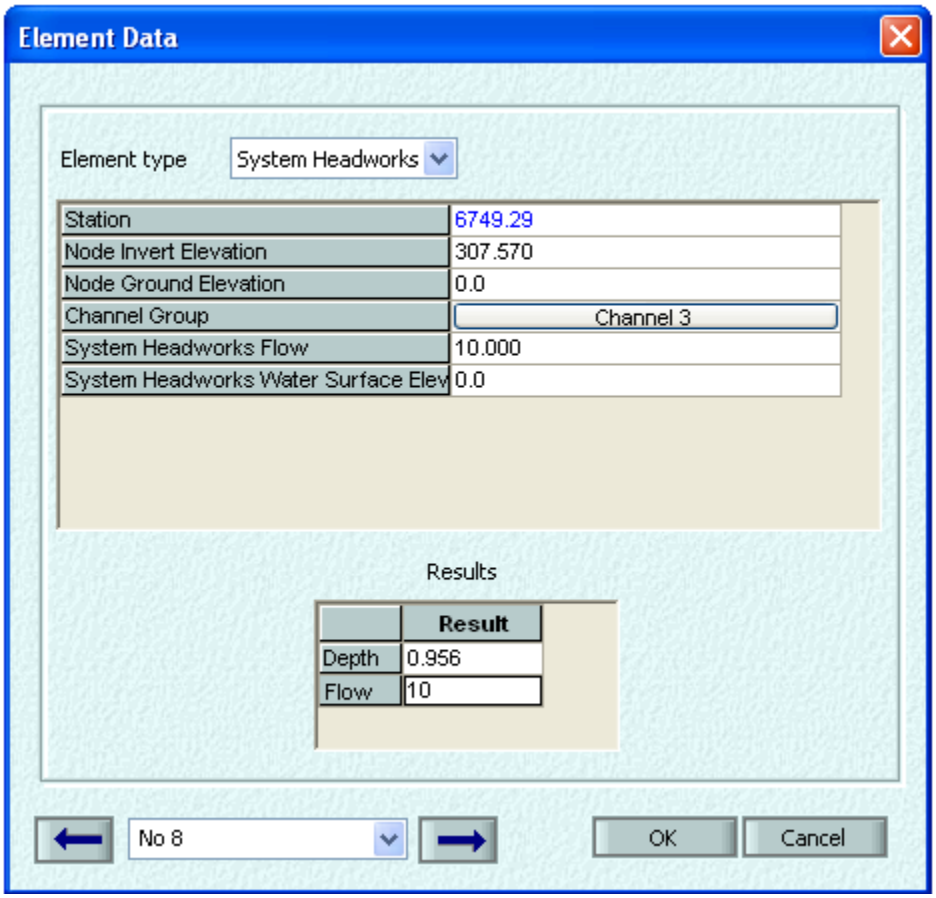


The Node Element data is used to specify the type of node from a pull down list and allows the user to input data associated with that node. Data input requirements vary based on the node selected. For reference, a quick view of the results for the selected element is displayed in the Element Data dialog, after the WSPG model has been solved.

The arrows progress the selection either upstream or downstream in the stream network.

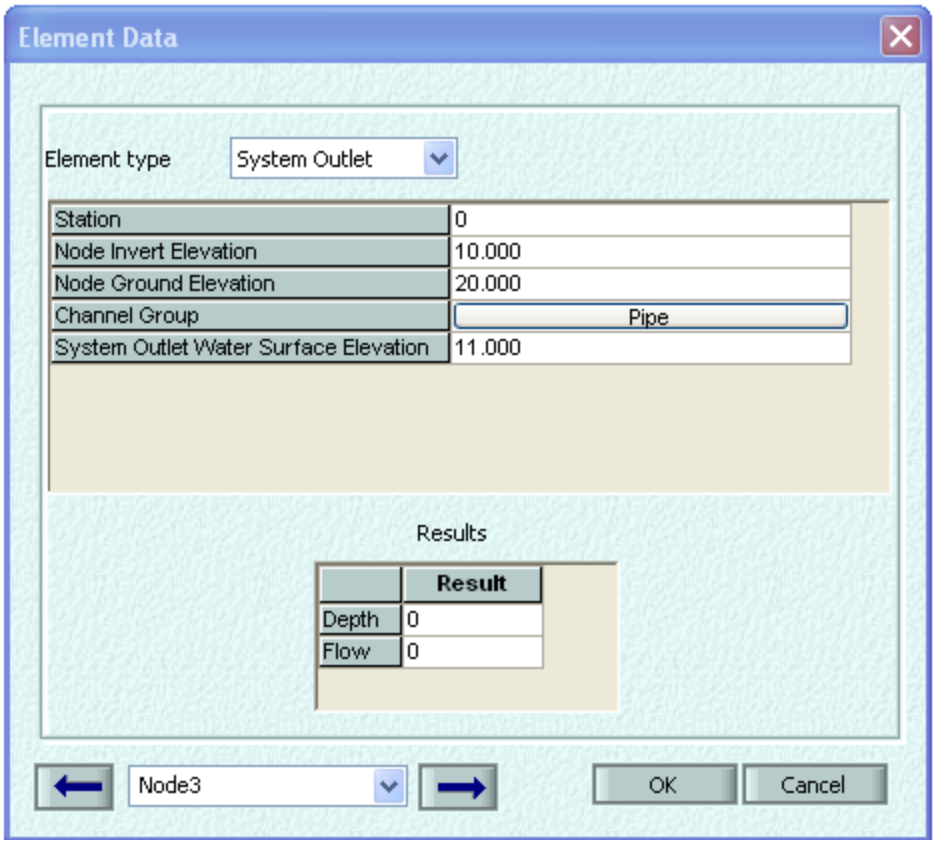
Station data can only be input at the System Outlet node. The program will automatically calculate stations for the remainder of the system based on the length of each subsequent element.

System Headworks



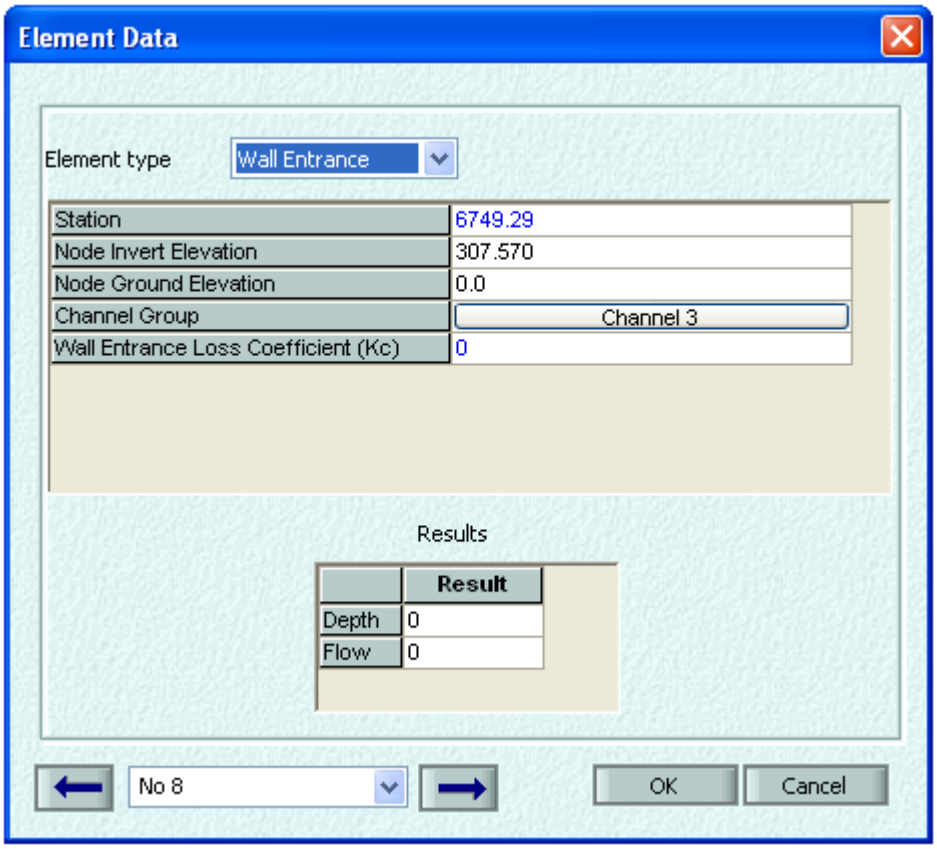
The System Headworks is always at the uppermost upstream element of a stream or branch network. The headwork represents the stream's inflow boundary condition. The flow rate is input as cubic feet per second (cfs). System headwork water surface elevation is in feet. The previous element's elevation and channel type must be used.

System Outlet



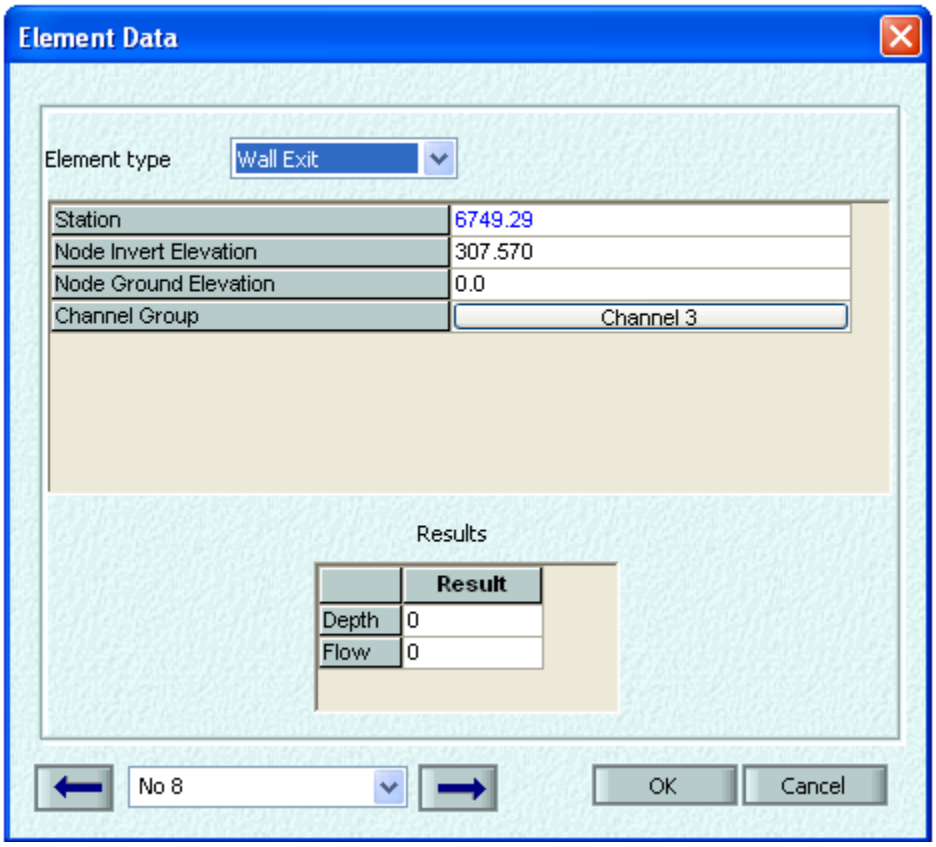
The System Outlet (SO) node represents the terminus of the stream network. Each WSPG model has exactly one outlet which is the most downstream element. There can be no further downstream elements. The outlet constitutes the outflow boundary condition. The Station is entered on the SO element data dialog. System Outlet Water Surface Elevation is in feet.

Wall Entrance



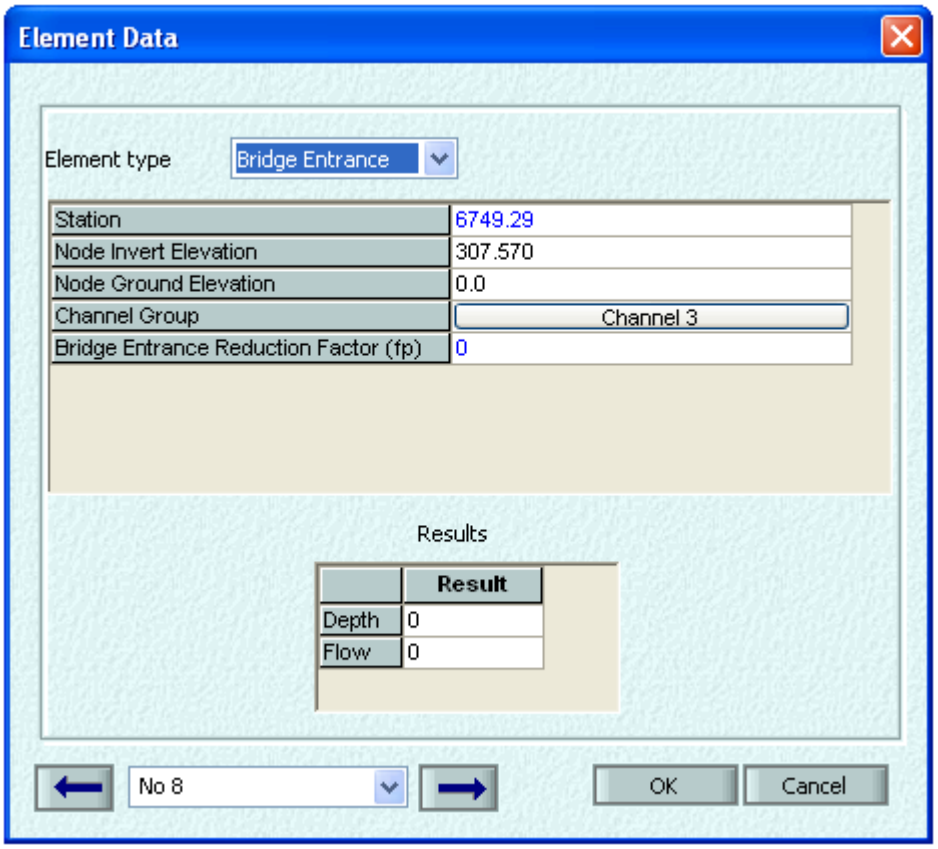
A Wall Entrance is used to model a sudden channel contraction. The channel upstream of the wall entrance cannot have piers. The entrance loss coefficient, Kc, value must be entered by the user, the default value is 0.5

Wall Exit



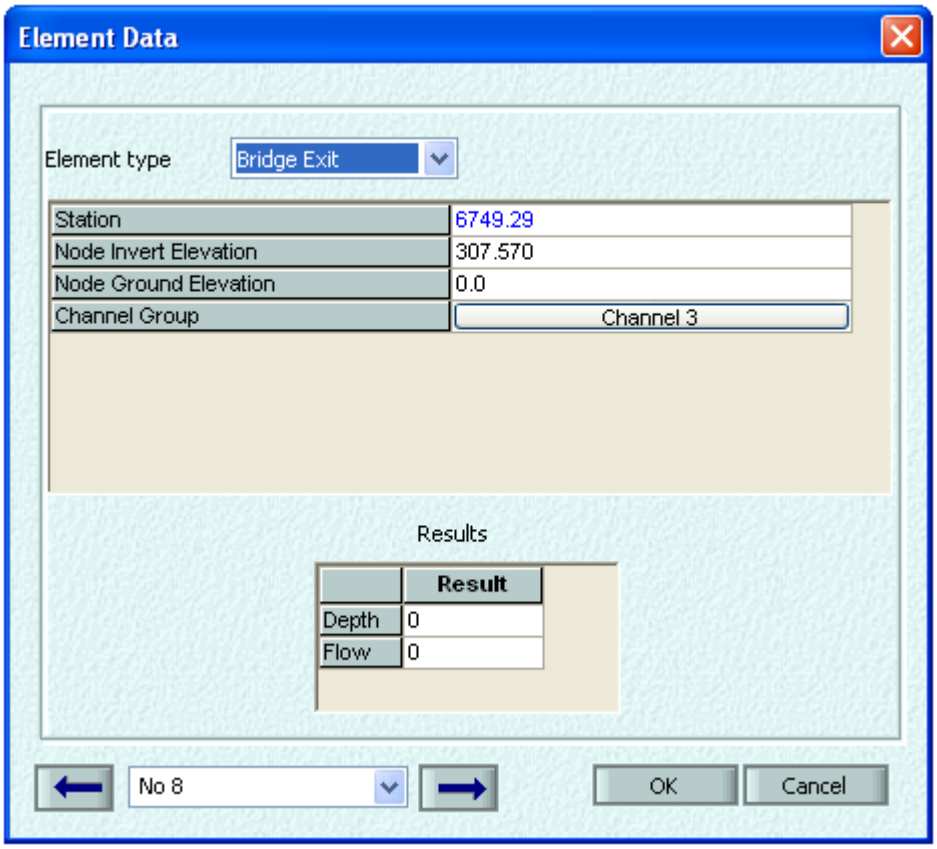
A wall exit is used to model a sudden channel expansion. The channel downstream the wall exit cannot have piers.

Bridge Entrance



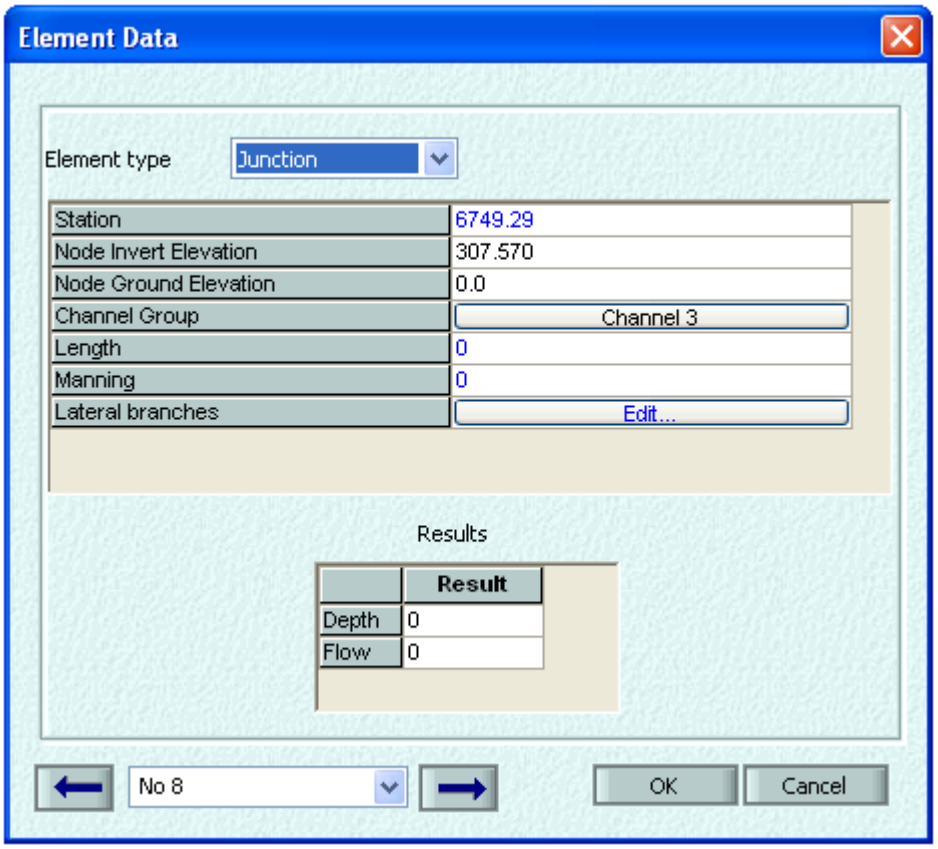
A Bridge Entrance element is used to transition from a section without piers to a section with piers. The Bridge Entrance Reduction Factor is a reduction factor for the area of the pier. The default reduction factor is 1.0.

Bridge Exit



The Bridge Exit element is used when a channel transitions from a section with piers into a section without piers.

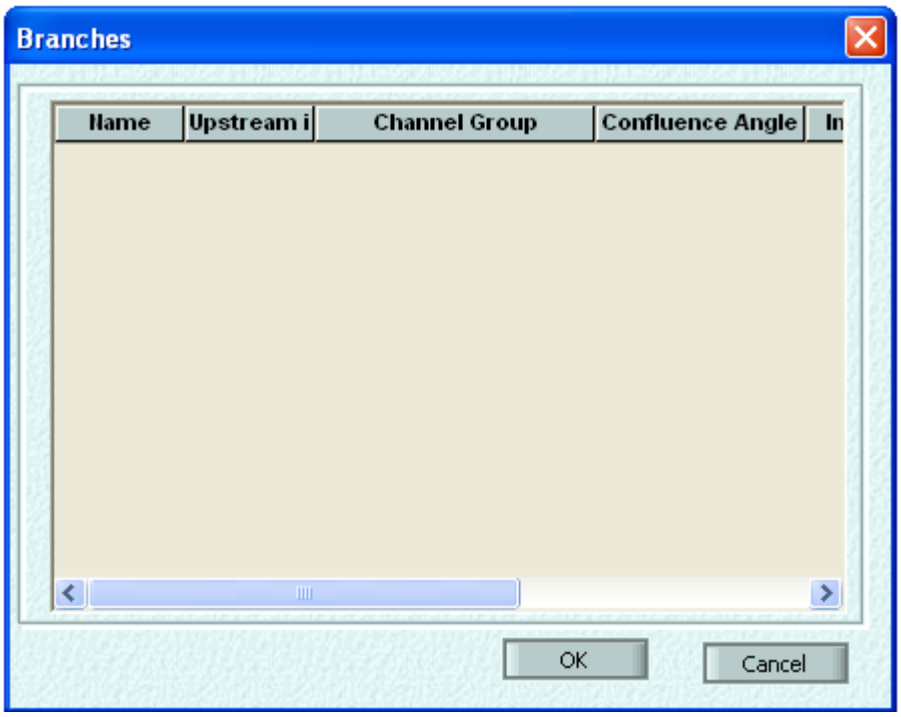
Junction



A Junction element is used to introduce a lateral flow, a side flow entering either a channel or pipe. The lateral invert elevation cannot be less than the downstream invert elevation. The junction cannot be bound by either a System Headworks or a System Outlet.

The lateral branch data must be input as well.

Lateral Branches

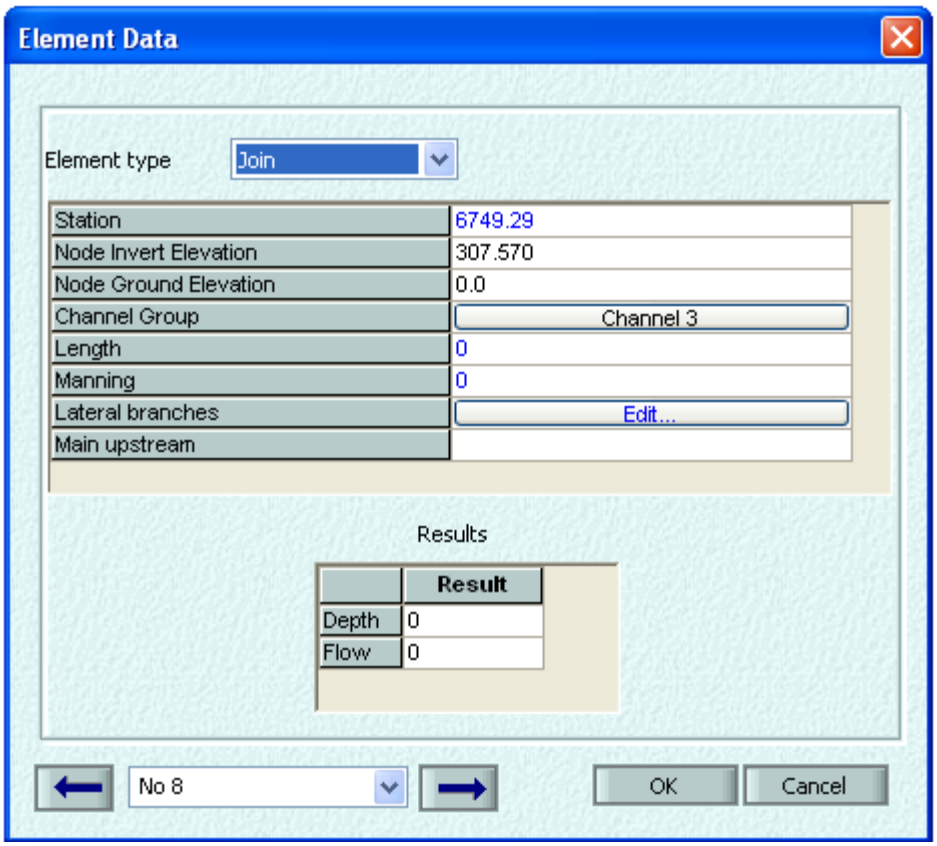


The Junction and Join elements require that a Lateral Branch be defined. The Confluence Angle (degrees) is used to calculate entrance losses. The incoming flow rate must be input (cfs). The invert elevation of the lateral branch cannot be less than the downstream invert.

Confluence angle must be between 0 and 90 degrees, either positive (+) or negative (-).

Lateral inflow may be negative.

Join

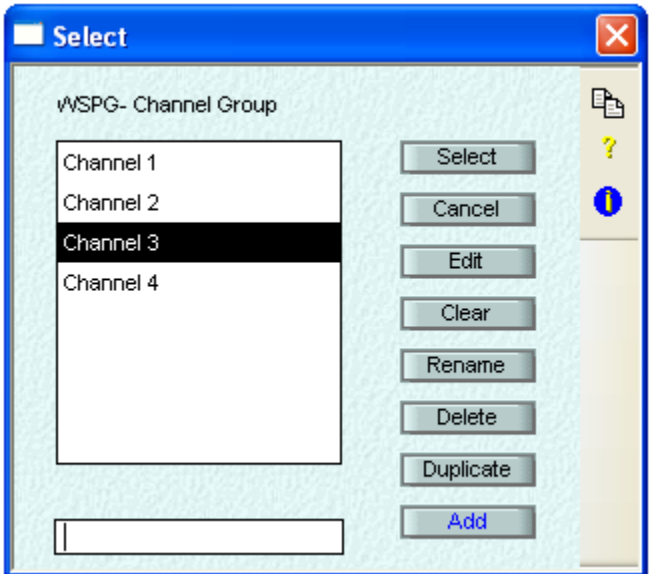


The Join Element is an extension of the Junction Element. The Join introduces a branch of the main stream. Rather than simply adding in flows from a branch, the system can be included in a single model. The branch system is defined similar to the main stream, with the exception that the downstream most element is a Join, rather than a System Outlet. The branch network must have at least one Reach and a System Headworks.

Joins can be adjacent to more than two link elements. For each Join, the mainstream line must be defined.

The program computes the main streamline first and then the branch. The program uses the water depth in the main streamline (from the composite profile) as an initial water surface elevation in branch network.

Channel Selection



The Channel Select dialog is used to select channel types to add to the network. The channel name is entered at the bottom, the Add button moves the channel name to the above list. The Edit button opens the WSPG Channel Type editor. Channels in the list may be used more than once or not used at all. The Select button returns the program to the Element Data dialog, where the channel name is displayed as the Channel Group.

Channel Types

WSPG channel types are defined as Types 1-6. The channel input provides the basic geometry upon which the hydraulic calculations are based. Refer to the April 1979 WSPG User Manual for more information.

Trapezoidal Open

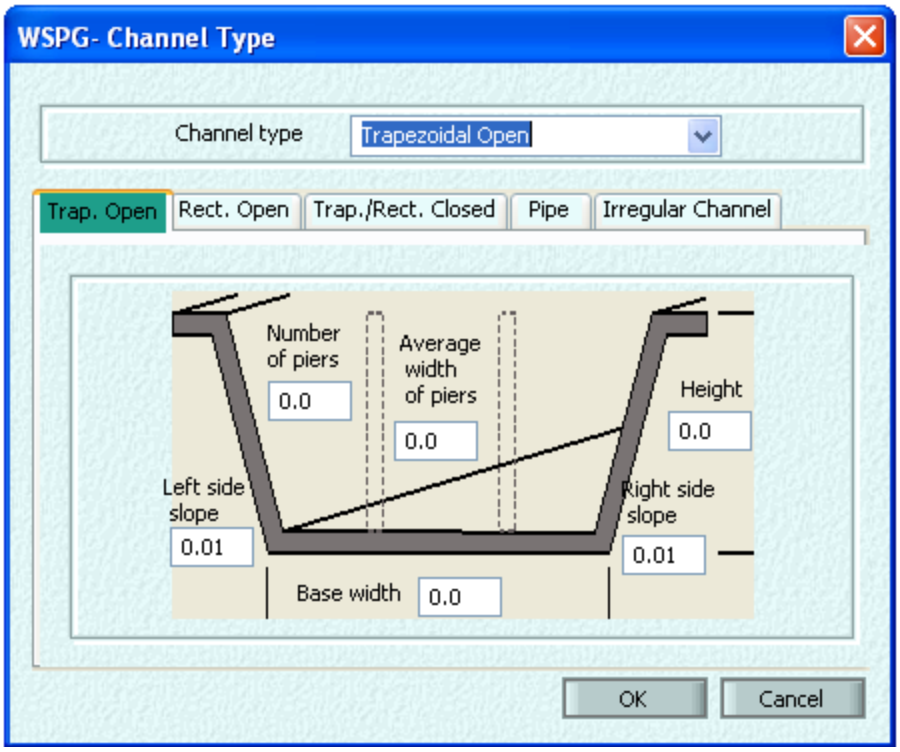
Rectangular Open

Trapezoidal / Rectangular Closed

Pipe

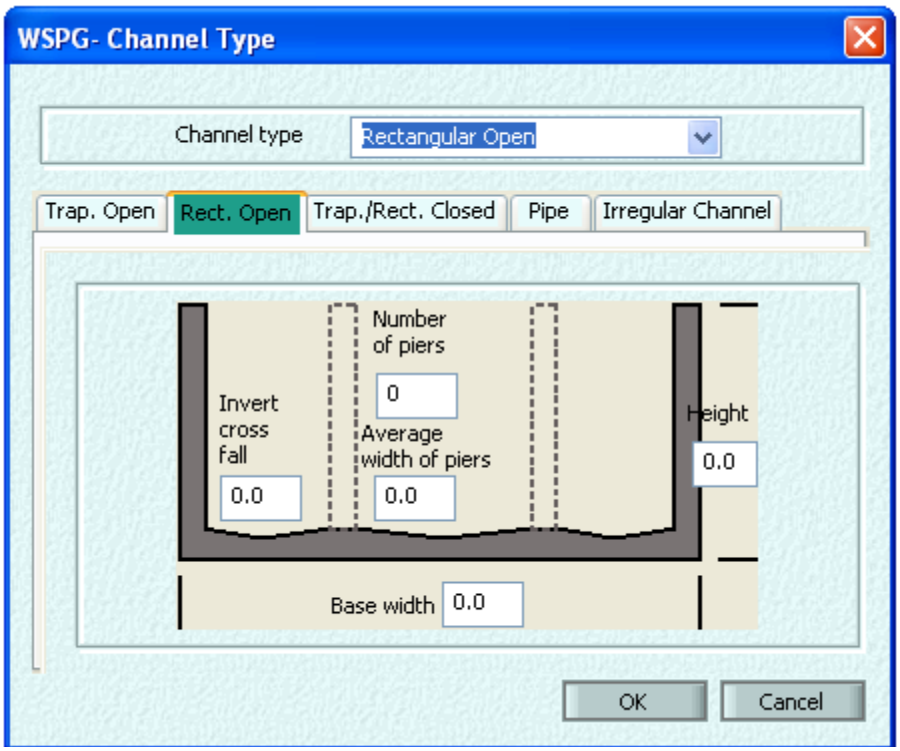
Irregular

Trapezoidal Open



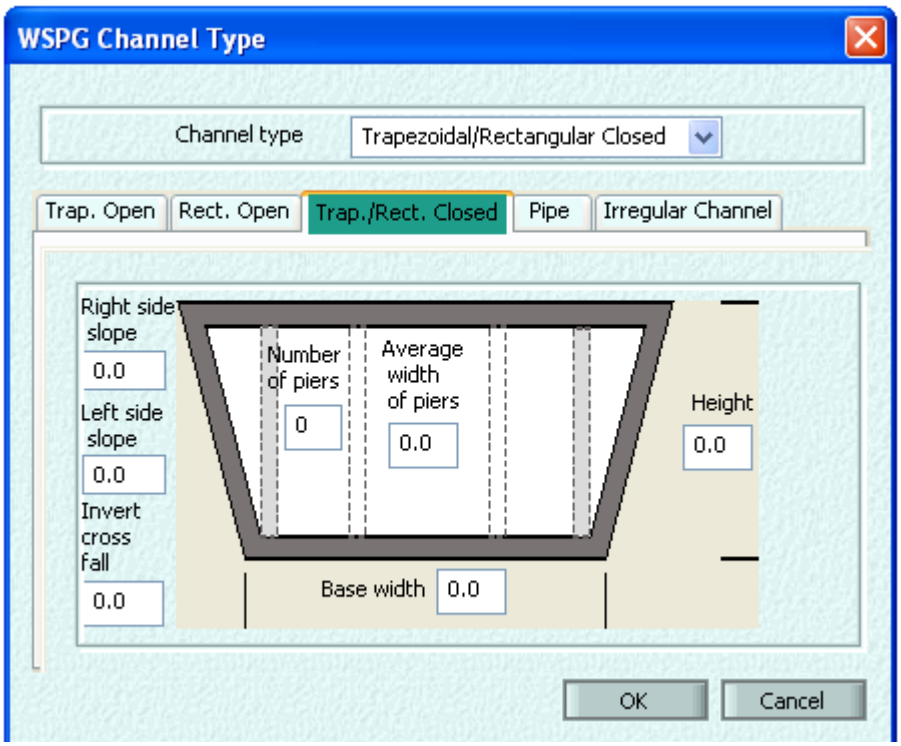
Number of Piers is optional; however, if greater than 0, then a value for the Average width of piers must be input. If the computed water depth is greater than the input Height value, the program will assume vertical walls extend from the ends of the cross section, indicating depth has exceeded the channel height. If piers are present, it is assumed that piers have infinite height.

Rectangular Open



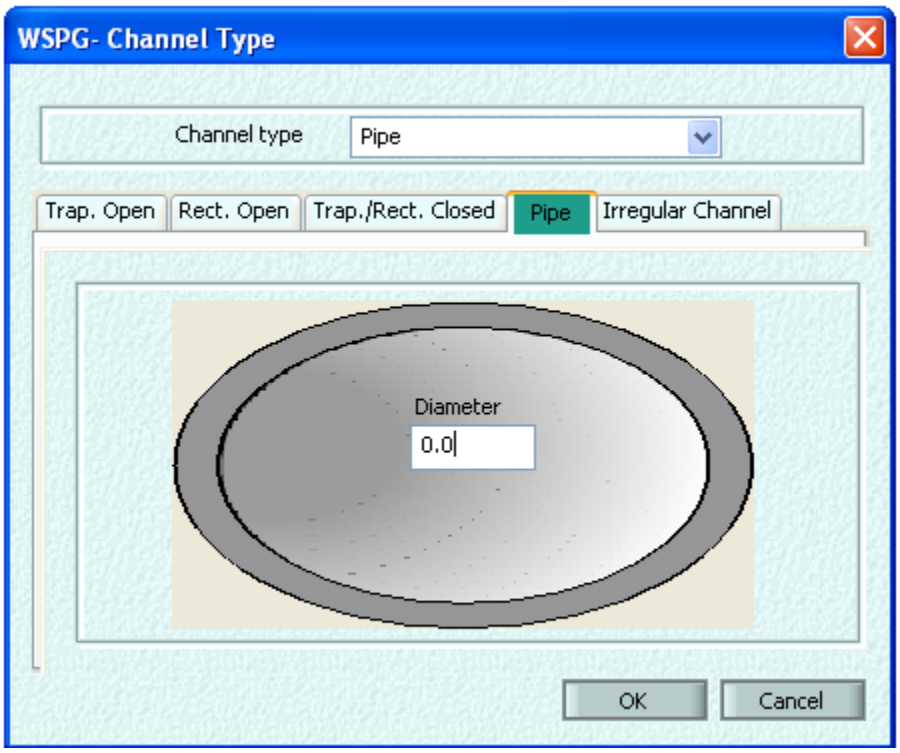
Number of Piers is optional; however, if greater than 0, then a value for the Average width of piers must be input. If the computed water depth is greater than the input Height value, the program will assume vertical walls extend from the ends of the cross section, indicating depth has exceeded the channel height. If piers are present, it is assumed that piers have infinite height.

Trapezoidal / Rectangular Closed



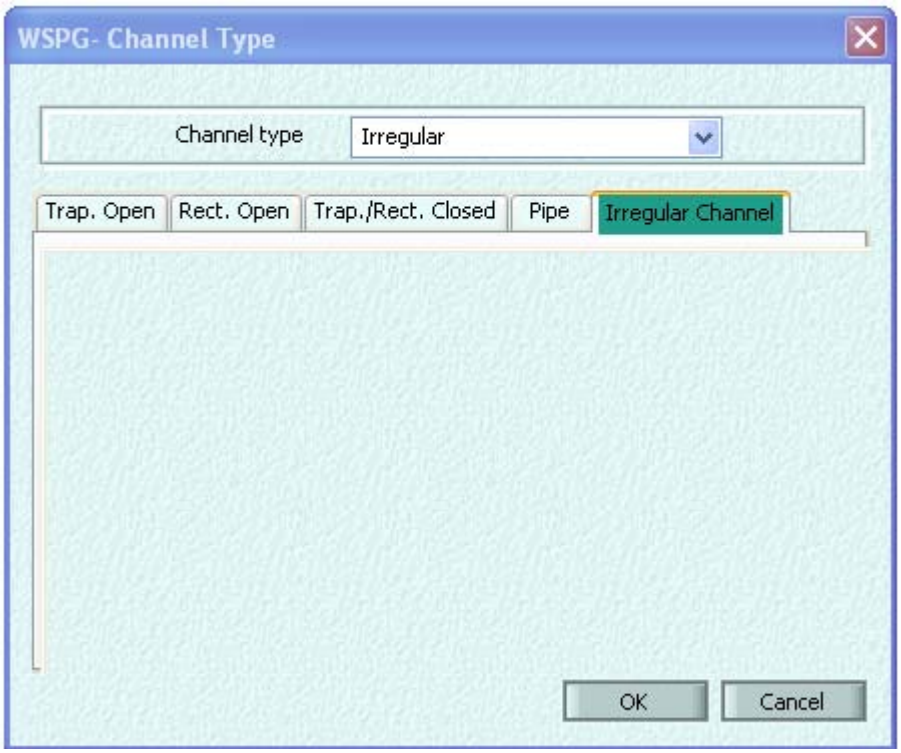
Number of Piers is optional; however, if greater than 0, then a value for the Average width of piers must be input. If the computed water depth is greater than the input Height value, the program will assume vertical walls extend from the ends of the cross section, indicating depth has exceeded the channel height. If piers are present, it is assumed that piers have infinite height.

Pipe



Circular pipe diameter is input in feet.

Irregular



Irregular channel opens the Irregular Channel editor.

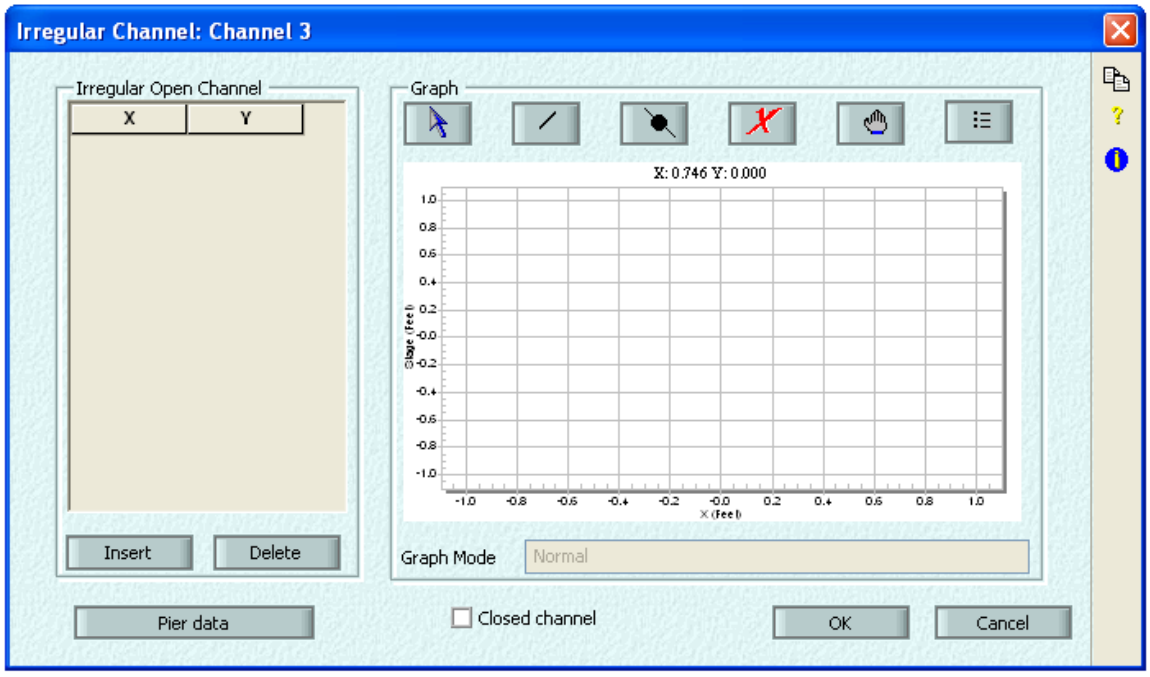
Restrictions were introduced on the point sets that characterize the cross section of an irregular channel. For an open irregular channel (type 5) the cross section has to be built by a non-strictly monotonously decreasing segment followed by a non-strictly monotonously increasing segment. This means that if following the cross section counter-clockwise, the x-coordinates have to come in a non-decreasing order and the y-coordinates are separable into a first sequence of non-increasing values and a second sequence of non-decreasing values.

Like in the original code, if the y-coordinates of the first and last point are not identical, the redundant part of the cross section will be truncated.

For an irregular closed channel (type 6) a point set that fulfills the requirements has to be built by four subsequent sequences with

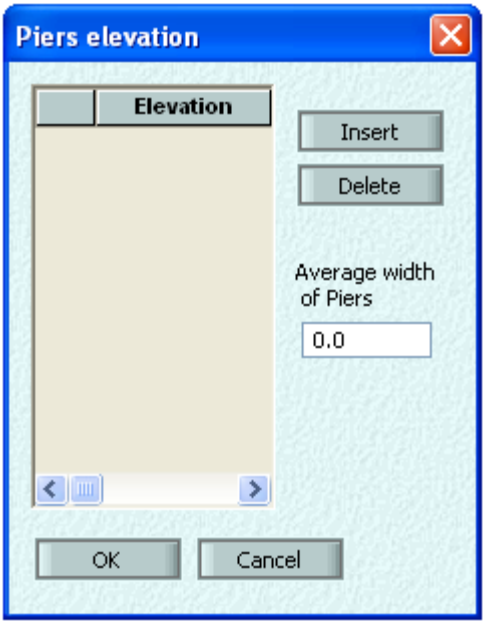
1. non-decreasing x-coordinates and non-increasing y-coordinates
2. non-decreasing x-coordinates and non-decreasing y-coordinates
3. non-increasing x-coordinates and non-decreasing y-coordinates
4. non-increasing x-coordinates and non-increasing y-coordinates.

Irregular channel editor



See the full XP help file for a complete description of the irregular channel editor.

Pier Elevations Input



The reference (x,y) axis for piers must be the same as those used for the cross section. The “y” values are given from left to right.

Warning Messages

Warning messages

WARNING 01: Insufficient lateral characterization for junction/join [x].

WARNING 02: Maximum number of laterals for junction/join [x] reached. Rest of data will be discarded.

WARNING 03: No branches given. Join [x] will be treated as a transition.

WARNING 04: Not all elements are connected to the outlet! Computation will only be performed on objects connected to the outlet.

WARNING 05: Too many parameters given for channel section [x]. Excessive parameters will be ignored.

WARNING 06: Upstream channel and downstream channel are the same for transition [x]. Use a reach instead?

WARNING 07: Node [x] has negative invert elevation.

WARNING 08: Number of manholes given for non-closed link [x].

WARNING 09: Pier factor for bridge entrance [x] not in valid range. Pier factor set to 1.0.

WARNING 10: Wall factor for wall entrance [x] not in valid range. Wall factor set to 0.5.

WARNING 11: Element [x] is not connected to the system outlet.

WARNING 12: Link [x] is not connected to the system outlet.

WARNING 13: Pier factor for bridge entrance [x] is below 0.667.

WARNING 14: Wall factor for wall entrance [x] is not between 0.2 and 0.9.

WARNING 15: Invert elevation of element [x] is not larger than invert elevation of the direct downstream element.

WARNING 16: One of the elevations of junction's [x] laterals is set to the invert elevation of the junction.

WARNING 17: One of the confluence angles of junction's [x] lateral is set to 90 degree.

WARNING 18: Length of junction [x] exceeds 100 feet. This is not recommended.

WARNING 19: Right slope in trapezoidal data of channel [x] set to 1.0.

WARNING 20: Left slope in trapezoidal data of channel [x] set to 1.0.

WARNING 21: A pier elevation in channel [x] is set to 0.0 because it exceeded the channel height.

WARNING 22: Element keyword [x] detected when reading a branch name.

WARNING 23: For junction/join [x] fewer branches have been defined than stated by the branch number. Only the defined branches will be processed.

WARNING 24: For junction [x] there are equally named branches.

WARNING 25: Link type element [x] has different invert elevation than its upstream node.

WARNING 26: (Not used)

WARNING 27: (Not used)

WARNING 28: (Not used)

WARNING 29: Normal depth for reach [x] could not be computed accurately. Value used in processing may be inaccurate.

WARNING 30: Internal error. Trying to compute friction slope for element [x] with a negative wetted area or perimeter. Set friction slope to 0.0.

WARNING 31: (Not used)

WARNING 32: Warning! Known depth equaled normal depth in Bernoulli computation for reach [x].

WARNING 33: Upper and lower limits in Bernoulli computation are the same.

WARNING 34: The target value in the Bernoulli computation is not between the bounds.

WARNING 35: (Not used)

WARNING 36: D/S processing stopped in junction [x] because critical momentum is greater than maximum momentum.

WARNING 36: (Not used)

WARNING 37: (Not used)

WARNING 38: U/S processing stopped in junction [x] because the downstream momentum is less than maximum momentum.

WARNING 39: Internal error during U/S processing of transition [x]. Pressure detected but computed depth is less than maximum flow depth. Processing proceeds with less than pressure flow depth.

WARNING 40: (Not used)

WARNING 41: Steep reach in U/S processing detected in which water depth would drop below critical depth. U/S processing stopped.

WARNING 42: (Not used)

WARNING 43: (Not used)

WARNING 44: Jump location in reach [x] cannot be computed.

WARNING 45: One of the elevations of junction's [x] laterals is less than the U/S elevation.

WARNING 46: Channel of junction [x] defined by an irregular point set.

WARNING 47: Junction [x] has a different channel than its direct downstream element.

WARNING 48: Manning's n for pipe flow of element [x] not in recommended range 0.010 - 0.015.

WARNING 49: Manning's n for open flow of element [x] not in recommended range 0.01 - 0.06.

WARNING 50: Upstream processing: Water depth exceeds maximum open channel width for element [x]. Vertical walls used for processing.

WARNING 51: Unknown keyword [x] encountered in [REPORT] section. Program will run with default option.

WARNING 52: Point given for regular channel [x]. Point will be discarded and program runs with regular channel data.

WARNING 53: Headwork [x] has a higher elevation than its direct downstream node.

Error Messages

Error messages 10, 24, 50 and 53 are currently not used

ERROR 1: Memory allocation error in WSPG engine.

ERROR 2: Given channel for object [x] does not exist.

ERROR 3: Element [x] seems to end a streamline but is not a headwork.

ERROR 4: Node [x] is a headwork and has an upstream node.

ERROR 5: Node [x] is the system outlet and has a downstream node.

ERROR 6: Second system outlet [x] given. (Only one system outlet allowed.)

ERROR 7: Object [x] has more than one downstream element.

ERROR 8: Trying to connect the same element [x] to a join twice.

ERROR 9: Object [x] has more than one upstream element.

ERROR 10: Please use ascending order 1,2,3,... for channel numbers. This identifier is invalid: [x].

ERROR 11: Piers defined for channel [x] but average pier width data is missing.

ERROR 12: Point set [x] requested in channel section not found.

ERROR 13: Invalid number of piers (<0 or >10) for channel section [x].

ERROR 14: Not enough pier elevations given for channel [x].

ERROR 15: Channel cross section is changing at node [x]. Must replace node by a WALL-/ or BRIDGE-type node.

ERROR 16: Upstream and downstream channels change for reach [x]. Replace by a transition?"

ERROR 17: Headwork [x] has a higher elevation than its direct downstream node.

ERROR 18: Flow defined at headwork [x] is non-positive.

ERROR 19: Channel type for channel [x] is irregular! Types have to be between 1 and 6.

ERROR 20: Link [x] has non-positive length.

ERROR 21: Invalid curvature for closed channel [x]. Absolute value of curvature must be between 0 and 180 degrees.

ERROR 22: Invalid angle point for closed channel [x]. Angle point must be between 0 and 15 degrees.

ERROR 23: Manhole number of channel [x] must be between 1 and 10.

ERROR 24: Invalid value for Manning's n for channel [x].

ERROR 25: Bridge element [x] cannot have a pipe as its downstream or upstream element. Either replace bridge element or channel definition.

ERROR 26: Channel types at bridge element [x] do not change.

ERROR 27: Channel cross sections of bridge elements [x] downstream and upstream channel do not match.

ERROR 28: Channel types at wall element [x] do not change.

ERROR 29: Upstream channel of wall entrance [x] contains piers. Use a bridge exit instead?

ERROR 30: Downstream channel of wall exit [x] contains piers. Use a bridge entrance instead?

ERROR 31: Junction [x] has a channel of irregular type.

ERROR 32: Point set for channel [x] exceeds maximum allowed number of 99.

ERROR 33: One or more errors were detected while reading the data file. Please correct the above errors first.

ERROR 34: Too many characters in input line.

ERROR 35: Input data incomplete. Too few items.

ERROR 36: Branch [x] is defined twice.

ERROR 37: Invalid keyword [x] in the data file.

ERROR 38: Duplicate ID name [x].

ERROR 39: Undefined object [x].

ERROR 40: Invalid number [x].

ERROR 41: Channel height of channel [x] is not positive.

ERROR 42: Channel width of channel [x] is not positive.

ERROR 43: Total pier width for channel [x] exceeds the channel width.

ERROR 44: Pier width for channel [x] must be a positive number.

ERROR 45: Irregular point set of channel [x] has less than 3 points.

ERROR 46: Irregular point set of channel [x] does not fulfill monotonous sequence requirements.

ERROR 47: Downstream element of element [x] is of invalid type. Element types do not match.

ERROR 48: Upstream element of element [x] is of invalid type. Element types do not match.

ERROR 49: Cannot find element [x]. Has the element been defined?

ERROR 50: Files share same names.

ERROR 51: Cannot open input file.

ERROR 52: Cannot open output file.

ERROR 53: Cannot open binary results file.

ERROR 54: There are more branches defined for junction/join [x] than stated by the branch number.

ERROR 55: [x] is larger than the maximum allowed branch number.

ERROR 56: Branch name [x] has already been used for the join.

ERROR 57: Branch name [x] given in the [NETWORK] section hasn't been assigned to any join.

ERROR 58: The [NETWORK] section is supposed to start with an outlet. It starts with element [x] which is not an outlet.

ERROR 59: The beginning of a branch is expected to start with a join. Element [x] found which is not a join.

ERROR 60: Flow specified at headwork [x] has not the same value than the flow specified at the corresponding join.

ERROR 61: Missing branch for join [x]. More laterals have been defined than branches were given.

ERROR 62: Outlet already used.

ERROR 63: No headwork expected [x], stream too short branch.

ERROR 64: The branch name [x] hasn't been assigned to the join.

ERROR 65: Error in computation! Negative wetted area computed for channel [x]. Abnormal stop of processing.

ERROR 66: Internal error when computing wetted area. Abnormal stop of processing.

ERROR 67: Error in computation! Negative wetted perimeter computed. Abnormal stop of processing.

ERROR 68: Error in computation! Negative depth computed. Abnormal stop of processing.

ERROR 69: Error in processing: No wall obstruction in wall entrance [x] detected. Abnormal stop of computation.

ERROR 70: Abnormal stop during U/S processing of element [x]. Water elevation too high for maximum channel depth."

ERROR 71: Couldn't solve quadratic equation when determining end of pressure flow in reach [x] during U/S processing.

ERROR 72: Abnormal stop during U/S processing of reach: water depth is too high for reach with non-positive slope.

ERROR 73: There was neither a D/S processing nor an U/S processing for element [x].

ERROR 74: Reading the old WSPG data file format failed.

ERROR 75: Junction/Join [x] has a lateral with invalid internal channel number.

ERROR 76: Junction/Join [x] has a lateral whose invert elevation is less than the D/S invert elevation.

ERROR 77: There were errors when validating the input data. Please correct the above errors first.

ERROR 78: Error when validating [x]. A valid channel description for the direct downstream element could not be found.

ERROR 79: Cannot open error log file.

ERROR 80: No system outlet detected.

ERROR 81: Channel [x] not found.

ERROR 82: Channel ID [x] defined multiple times.

ERROR 83: Error in computation! Pier width exceeds top flow width for channel [x]. (Wetted area set to 0.0.)

Index

B

Basic Theory 3
Boundary Lines 7
Bridge Entrance 31
Bridge Entrance BE 13
Bridge Exit 32
Bridge Exit BX 14

C

Channel and Conduit Section Description 19
Channel Selection 36
Channel Types 37
Computational Procedures 3
Critical and Normal Depths 4

D

Data Input Descriptions 23
Data Processing System Description 5
Definitions & Restrictions for Irregular Sections 21
Distributed Versions of WSPG 1

E

Element Types and Construction 7
Error Messages 49

F

Flow Rates 3
Forward 1

I

Introduction 3
Introduction to WSPG 1
Irregular 41
Irregular channel editor 42
Irregular Sections 20

J

Join 35
Join Structure JO 12
Junction 33
Junction Structure JX 11

L

Lateral Branches 34
Link Element Data Dialog 23

M

Manning's 3

N

Node Element Data 26

P

Pier Elevations Input 43
Pipe 40
Purpose of WSPG 1

R

Reach 24
Reach R 10
Rectangular Open 38
Regular Channel Type Sections 19

S

Starting Water Surface Elevations 3
System Headworks 27
System Headworks SH 9
System Outlet 28
System Outlet SO 8

T

Transition 25
Transition Structure TS 12
Trapezoidal / Rectangular Closed 39
Trapezoidal Open 37

V

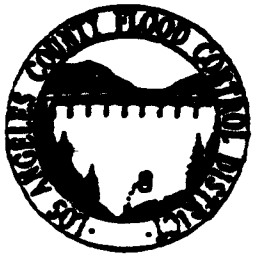
Velocity Head 4

W

Wall Entrance 29
Wall Entrance WE 15
Wall Exit 30
Wall Exit WX 16
Warning messages 45
Water Surface Stages 4
WSPG Job Control 23

WATER SURFACE PRESSURE GRADIENT

HYDRAULIC ANALYSIS COMPUTER PROGRAM FO515P USER MANUAL



APRIL 1979

COMPUTER PROGRAM F0515P

W S P G

Water Surface and Pressure Gradient

Hydraulic Analysis System

USER MANUAL

April 1979

Los Angeles County Flood Control District
A. E. Bruington, Chief Engineer

2250 Alcazar Street, Los Angeles
Telephone: (213) 226-4111

P.O. Box 2418 Terminal Annex
Los Angeles, CA 90054

W S P G

Water Surface and Pressure Gradient

Hydraulic Analysis System

USER MANUAL

Prepared By: Saman J. Shahin,
Garvin J. Pederson
Design Systems & Standards Group
Design Division

and

Richard A. Schaeffer
Business and Fiscal Division

April 1979

FORWARD

This program was written for use by the Los Angeles County Flood Control District or by its Contractors on District projects.

The use of this program by others is made and accepted with the understanding the Los Angeles County Flood Control District makes no warranties, express or implied, concerning its accuracy, completeness, reliability, usability or suitability and the District shall be under no liability for any use made thereof.

This program is the property of the District. Therefore, a recipient further agrees not to assert any proprietary rights to this program or modification of or to represent it to anyone as other than a District program.

ACKNOWLEDGMENTS

Special thanks to Design Division Stenographers Deborah Mount and Rita Lopez for their patience and skill in typing this report. Thanks also to Eddie Nishiyama and Roy Fujimoto for preparing the drawings.

TABLE OF CONTENTS

SECTION	Page
1. Introduction	1
2. Purpose of Program	1
3. General Program Description	1
3.2 Basic Theory	1
3.3 Computational Procedure	1
3.3.1 Input Preparation	1
3.3.2 Flow Rates	2
3.3.3 Multiple Profiles	2
3.3.4 Manning's "n"	2
3.3.5 Water Surface Controls	2
3.3.6 Critical and Normal Depths	2
3.3.7 Water Surface Stages	3
4. Data Processing System Description	4
5. Element Description	5
5.1.1 Boundary Lines	5
5.1.2 System Outlet (SO)	5
5.1.3 System Headworks (SH)	5
5.1.4 Reach (R)	5
5.1.5 Junction Structure (JX)	6
5.1.6 Transition Structure (TS)	6
5.1.7 Bridge Entrance (BE)	6
5.1.8 Bridge Exit (BX)	6
5.1.9 Wall Entrance (WE)	7
5.1.10 Wall Exit (WX)	7

Table of Contents
Continued.

SECTION		PAGE
6.	Channel and Conduit Section Description	14
6.1.1	Regular Channel Sections	14
6.1.2	Irregular Sections	14
6.1.3	Definitions and Restrictions for Irregular Sections	15
7.	Input Data Description	18
7.1.1	Title Cards (T)	19
7.1.2	Element Cards	21
	System Outlet (SO)	22
	Reach (R), Channel Type 1, 2, or 5	23
	Reach (R), Channel Type 3, 4, or 6	24
	Transition Structure (TS)	25
	Junction Structure (JX)	26
	Bridge Entrance (BE)	27
	Bridge Exit (BX)	28
	Wall Entrance (WE)	29
	Wall Exit (WX)	30
	System Headworks (SH)	31
7.1.3	Channel Definition Cards	32
	(CD) Cards, Regular	33
	(CD) Cards, Irregular	34
7.1.4	PTS Card, x and y Coordinates	35
	(PTS) Card	36

Table of Contents
Continued.

SECTION PAGE

7.1.5 Q Cards ——— 38

7.1.6 Control Cards ——— 40

7.2 Sample Input ——— 41

8. Output Data Description ——— 50

8.1.1 Normal Output ——— 50

8.1.2 Optional Output ——— 50

8.1.3 Sample Output ——— 51

9. Error Messages ——— 70

9.1.1 Error Messages in Editing the Element Cards ——— 71

9.1.2 Warning Messages in Editing the Element Cards ——— 73

9.1.3 Error Messages in Sequence Checking thru Channel Definition Data ——— 74

9.1.4 Error Messages in Sequence Checking Cross Section Points Cards ——— 76

9.1.5 Error Messages in Channel Definition Processing ——— 78

9.1.6 Error Messages in Cross Section Point Processing ——— 79

9.1.7 Computation Error and Warning Messages ——— 80

9.1.8 Debug Message ——— 87

10. System Recovery Procedure ——— 88

11. Limitations ——— 89

12. Appendix ——— 90

1.1 Introduction

This program is a hydraulic analysis system developed by the Design Systems and Standards Group of the Design Division and the Data Processing Section of the Business and Fiscal Division of the Los Angeles County Flood Control District.

2.1 Purpose

The program computes and plots uniform and nonuniform steady flow water surface profiles and pressure gradients in open channels or closed conduits with irregular or regular sections. The flow in a system may alternate between super critical, subcritical or pressure flow in any sequence. The program will also analyze natural river channels although the principle use of the program is intended for determining profiles in improved flood control systems.

3.1 General Program Description

3.2 Basic Theory

The computational procedure is based on solving Bernoulli's equation for the total energy at each section and Manning's formula for friction loss between the sections in a reach. The open channel flow procedure utilizes the standard step method. Confluences and bridge piers are analyzed using pressure and momemtum theory.

The program uses basic mathematical and hydraulic principles to calculate all such data as cross sectional area, wetted perimeter, normal depth, critical depth, pressure, and momentum.

3.3 Computational Procedure

3.3.1 Input Preparation

The channel or conduit system is initially subdivided into the following elements: system outlet, reach, transition, confluence (junction), bridge exit, bridge entrance, wall entrance (sudden contraction), wall exit (sudden expansion), and system headworks. Each element is internally assigned a number. The input data must consist of a minimum of three elements (system outlet, system headwork and any other element) and is limited to a maximum of 200 elements. A greater number of elements will require a breakup into two or more systems.

3.3.2 Flow Rates

The starting flow rate (Q) at the upstream terminus of a system is specified on a "Q" card. The flow rate (Q) is increased at the desired locations by specifying lateral inflow rates on the "JX" cards. The flow rate can be reduced by using a negative lateral Q, this reduction is intended to account for channel storage. If it is used in cases where the channel or conduit branches it should be understood no loss is computed.

3.3.3 Multiple Profiles

To obtain additional watersurface or pressure gradient profiles for different flow rates in the system, additional Q cards may be supplied. The only limitation on the number of profiles that may be run at one time is the limit on the program execution time which is set by the computer center.

3.3.4 Manning's "n"

The program uses the Manning formula for the friction loss in all types of conduits or natural channels. The program can only take one "n" value per element, however, the "n" value can change at subsequent elements. If a section has a lining composed of different roughness coefficients a composite "n" based on anticipated depth of flow should be hand computed. If an "n" value is not specified with the input data, the program uses a value of .014.

3.3.5 Water Surface Controls

Water surface controls at the downstream terminus (System Outlet S.O.) or the upstream terminus (System Headworks S.H.) are optional input values. If water-surface controls are not given the program will use critical depth controls.

3.3.6 Critical and Normal Depths

Critical depth is computed for every section for the given Q utilizing the "Specific Energy Equation".

Normal depth is computed in every reach element on a positive slope for the specified Q.

The velocity head (H_v) is computed using the mean velocity of the section. This may not be accurate in the case of a complex section such as one with shallow flow in the horizontal overbank area where velocity distribution is not uniform. If the program is to be used in this situation the user should be aware that some error may be introduced in the results. A check on the magnitude of the error can be made by the user utilizing the parabolic method to determine specific energy (see Appendix).

3.3.7 Watersurface Stages

The lower stage w.s. profile begins at the system headworks and ends at the system outlet. The computation will proceed downstream in every consecutive element as long as energy is available to maintain flow in the supercritical stage. When energy becomes expended at any point in an element, the lower stage profile will be discontinued from that point to the downstream end of that element. Then computation will resume in the next element with a critical depth control until the system outlet is analyzed.

The upper stage w.s. profile, begins at the system outlet, and ends at the headworks. Computation proceeds upstream in every element as long as the water surface at the downstream end of any two adjacent points can support the moving mass of water to flow at the critical or subcritical depth. Otherwise, computation will be discontinued from the downstream point to the upstream end of that element. Then computation will resume at the downstream end of the next element with critical depth control, provided no depth less than critical depth has been computed at that point on the lower stage profile. Then computation will proceed upstream until the system headworks is analyzed. Note that if the computed depth of flow in any open section exceeds the given section height the program will assume an additional 10-feet of vertical wall except for Channel Type 1 (see Figure 6-1) where the side slopes are extended outward until the 10-feet vertical height is reached.

The jump routine begins at the system outlet and ends at the headworks. It searches the lower stage and the upper stage profiles for points of equal energy. If a jump is encountered, it will be approximately located; and data on either the upper stage or lower stage not consistent with the greater energy theory will be deleted from every element. The final profile will be a composite of upper stage and lower stage with hydraulic jumps in between.

4.1 Data Processing System Description

All programs are written in FORTAN IV, compiled using the IBM FORTRAN H compiler executing on an IBM 370/158 using OS/VS2 MVS. The system requires the use of an input media (such as a card reader), temporary disk storage, and a printer. It is designed to run in batch mode.

Required input to the system consists of:

1. Title information.
2. Channel element definitions.
3. Cross section definition.
4. Cross section points definition.
5. Q card, of which the first is required and the subsequent Q cards are optional for change of flow rate in the system.

The entire input is thoroughly scanned for required information and range values of optional information before processing begins. If any errors are detected, processing will stop. Warnings may be issued, but they will not prevent processing.

Processing consists of three phases: Analysis of the system in the downstream direction (phase I), analysis of the system in the upstream direction (Phase II), and analysis of the downstream profile (from Phase I) and the upstream profile (from Phase II) to obtain a composite profile (Phase III). The processing was designed to continue calculating unless gross errors are encountered. Warning messages may be issued concerning tolerance levels not being reached on an iterative approximation. These may or may not effect the overall solution to the problem; however, processing continues. If gross errors are encountered, an error message will be issued and processing will stop.

Output of the system consists of three reports:

1. A listing of input with edit scanning messages.
2. A w.s. profile listing of the composite profile obtained in Phase III of processing.
3. A profile plot of watersurface and channel geometry.

5.1 Element Description

The channel, conduit or natural river system to be analyzed is subdivided into elements as stated in Section 3.3.1. The program internally numbers the elements beginning with the System Outlet (SO) as number 1. Each successive element is numbered continuing upstream to the System Headworks (SH). The maximum number of elements permissable by the program is 200.

5.1.1 Boundary Lines (Refer to Figure 5-1)

All elements are bounded on the upstream end by Section 1 and the downstream end by Section 2 except System Outlet (SO) and System Headworks (SH) which only have Section 1. The user inputs data such as base width, conduit height, etc. for Section 1 of every element. The data for Section 2 for every element is taken by the program from the upstream Section 1 of the adjacent downstream element. Elements may have considerable length between Section 1 and Section 2 as in a reach element or may have a zero length as in a bridge entrance element.

L = length of element

X = number of the element under consideration

X+1 = adjacent upstream element

X-1 = adjacent downstream element

5.1.2 System Outlet (SO) (Refer to Figure 5-2)

The system outlet is the downstream terminus of a channel. X is equal to one. X+1 can be any element except a System Headworks (SH). Note the element length is zero.

5.1.3 System Headworks (SH) (Refer to Figure 5-3)

The system headworks is the upstream terminus of a channel. Element X-1 can be any element except a system outlet. Note the element length is zero.

5.1.4 Reach (R) (Refer to Figure 5-4)

The reach element is a length of channel, drain or natural river with a constant invert slope, Q, cross section and Manning's n. A reach may have a straight or curving horizontal alignment, however, a curved reach must coincide with the beginning and end of the curve. The same applies to an angle point in the horizontal alignment, a reach must end or begin at the angle point.

In open channels (regular rectangular or trapezoidal sections) the superelevation of the watersurface is computed and printed for each point in the curve. In pressure flow, bend losses, angle point losses, and manhole losses are computed and added to the friction loss for the reach.

Element X+1 can be any element except a system outlet.
Element X-1 can be any element except a system headworks.

5.1.5 Junction Structure (JX) (Refer to Figure 5-5)

The junction structure element is used where there is lateral inflow into the system. Two different laterals can be handled by this element. Element X-1 can be any other element except a System Headworks (SH). Element X+1 can be any other element except a System Outlet (SO).

5.1.6 Transition Structure (TS) Refer to Figure 5-6

A transition structure is a gradual expansion or contraction from Section 1 to section 2. The length L may be any positive number. Element X+1 may be any element except a system outlet. (SO) Element X-1 may be any element except a system headworks (SH).

5.1.7 Bridge Entrance (BE) (Refer to Figure 5-7)

A bridge entrance is an element used where flow enters from an element without piers into an element with piers.

A bridge entrance is considered to have a zero length element even though the bridge pier nose may have a minor length.

Element X-1 may be a SO, R, JX or TS.
Element X+1 may be a R, JX, TS or SH.
It is noted that neither section 1 or 2 can be a pipe.

5.1.8 Bridge Exit (BX) (Refer to Figure 5-8)

The bridge exit is also considered to have a zero length element.

A bridge exit is an element used where flow exits from an element with piers into an element without piers.

Element X-1 may be a SO, R, JX, or TS.
Element X+1 may be a SH, R, JX, or TS.
It is noted that neither section 1 or 2 can be a pipe.

5.1.9 Wall Entrance (WE) (Refer to Figure 5-9)

This element is used when there is a sudden change in the conduit section such as a headwall or an abrupt contraction. This element is considered to have a zero length.

The user should supply the loss coefficient Kc expressed in terms of the velocity head. If left blank on the input card the program uses a value of .5 for kc. (See Hydraulic Handbooks for typical values).

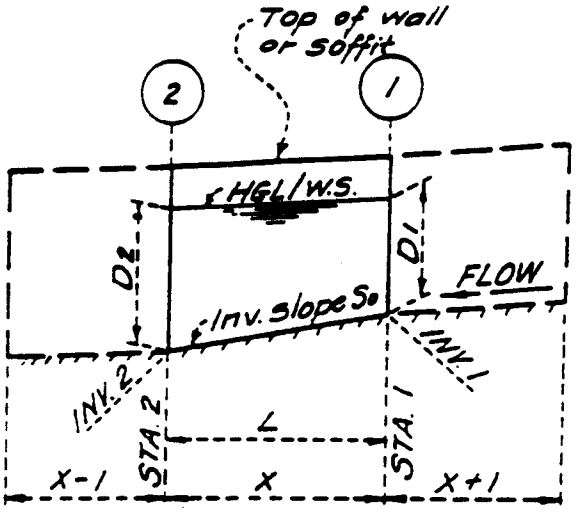
Element X-1 may be a SO, R, JX, or TS.
Element X+1 may be a SH, R, JX, or TS.
The section for element X+1 cannot have piers, however, it can be an open channel or closed conduit. The section for element X-1 can also be an open channel or closed conduit and it can be with or without piers.

5.1.10 Wall Exit (WX) (Refer to Figure 5.10)

This element is used when there is a sudden expansion from a smaller to a larger channel or conduit section. This element is considered to have a zero length.

Element X-1 may be a SO, R, JX or TS.
Element X+1 may be a SH, R, JX or TS.
The section for element X+1 may be an open channel or closed conduit with or without piers. The section for element X-1 may be an open channel or closed conduit however it cannot have piers.

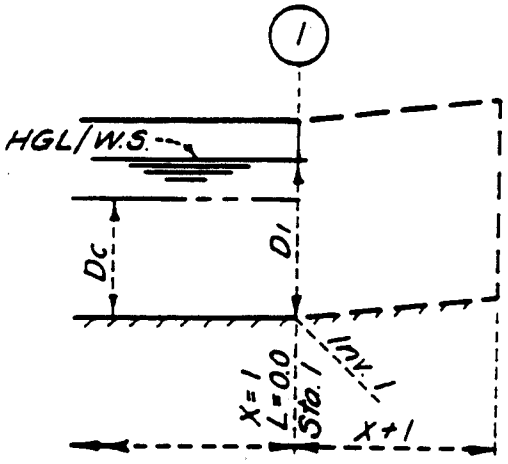
ELEMENT BOUNDARY LINES



ELEVATION

FIG. 5 - 1

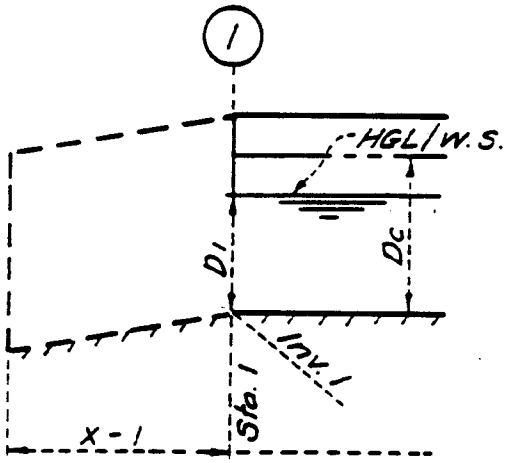
SYSTEM OUTLET



ELEVATION

FIG. 5-2

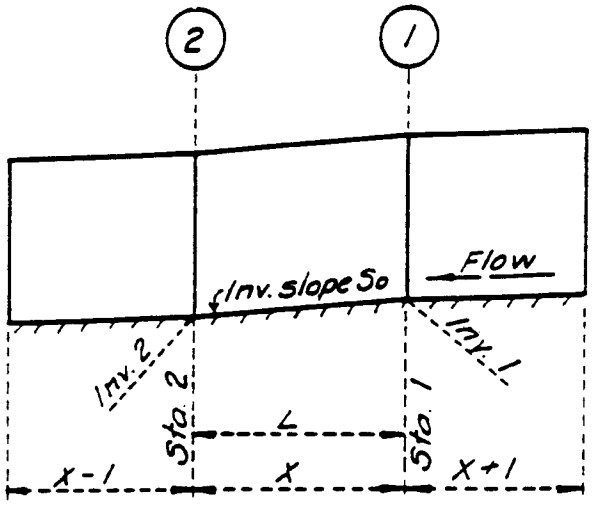
SYSTEM HEADWORKS



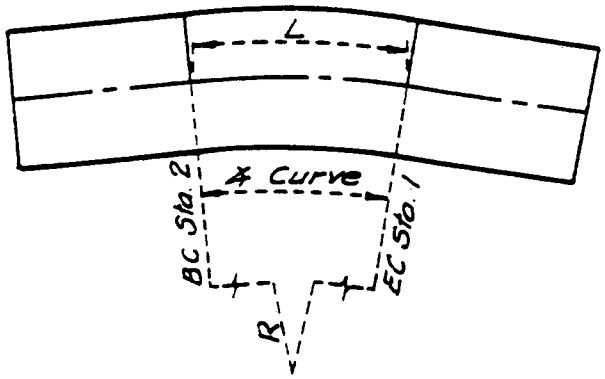
ELEVATION

FIG. 5-3

REACH

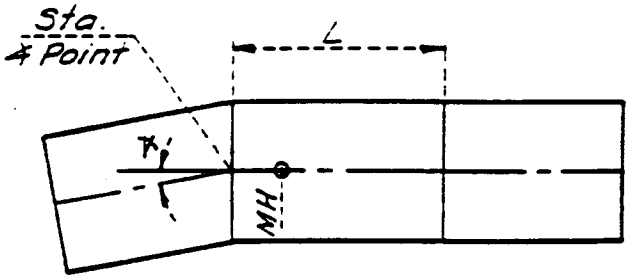


ELEVATION



PLAN A

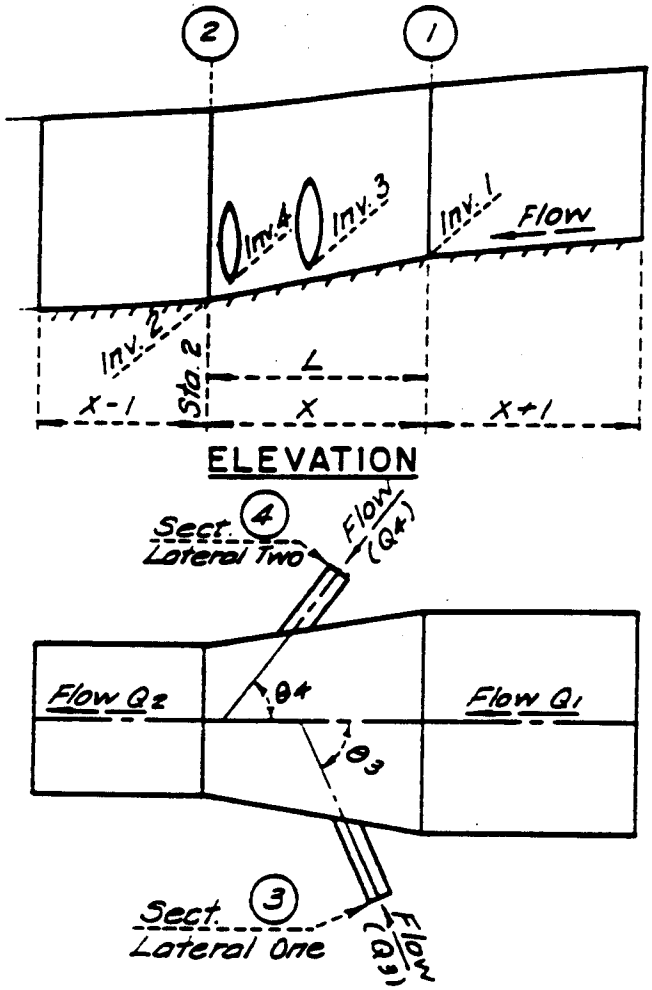
(EXAMPLE OF CURVED ALIGNMENT)



PLAN B

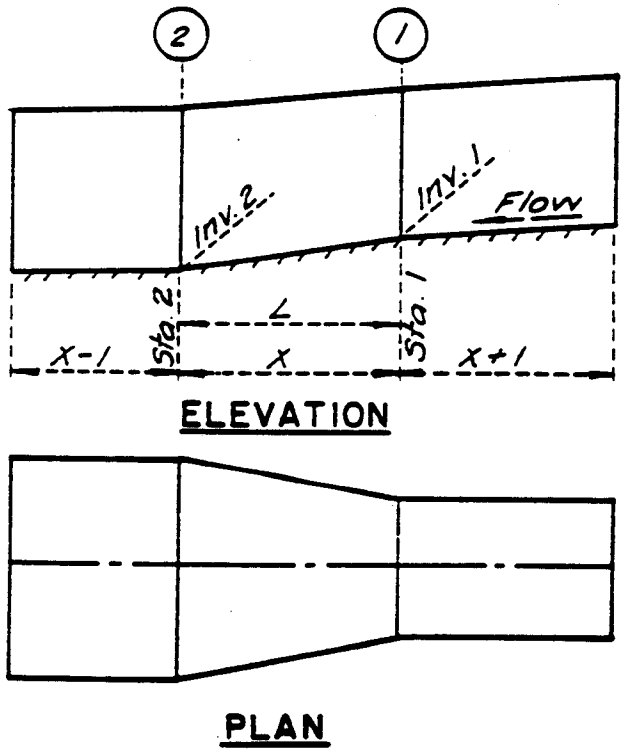
(EXAMPLE OF A STRAIGHT REACH WITH ℄ PT. AT D/S END)

FIG. 5-4



JUNCTION

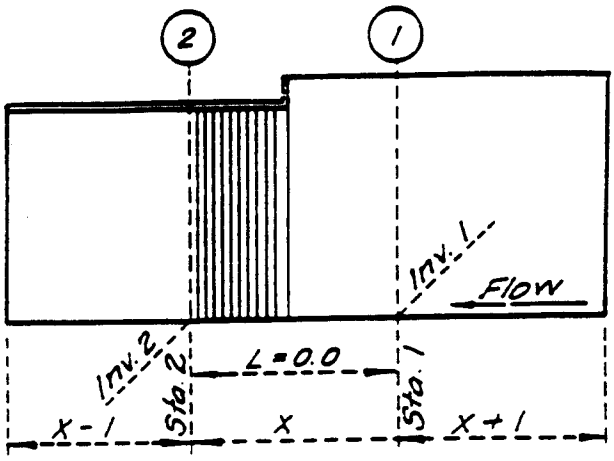
FIG. 5-5



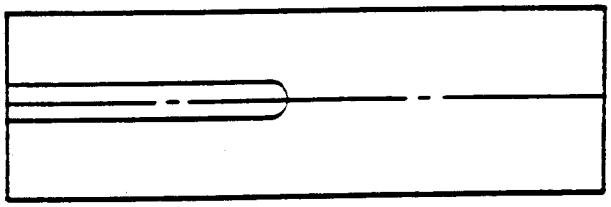
TRANSITION

FIG. 5-6

BRIDGE ENTRANCE



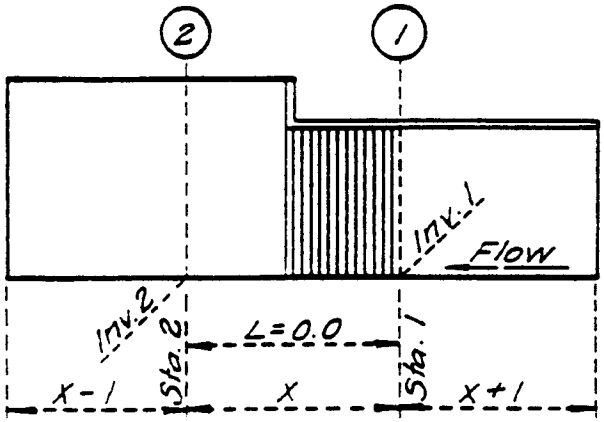
ELEVATION



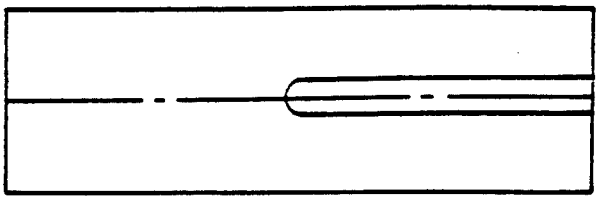
PLAN

FIG. 5-7

BRIDGE EXIT



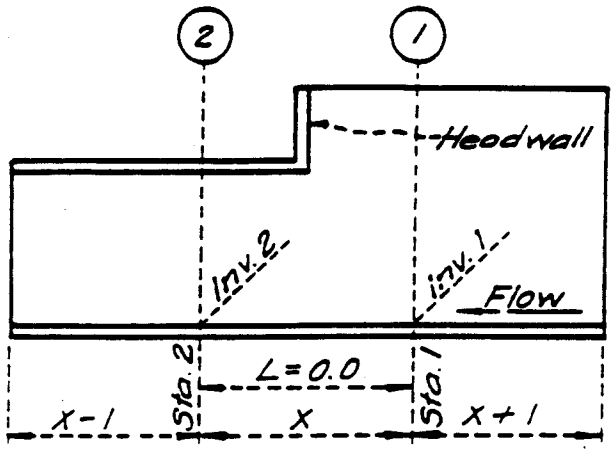
ELEVATION



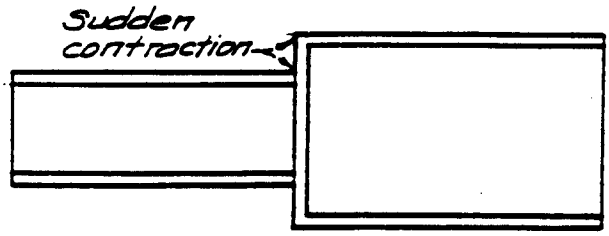
PLAN

FIG. 5-8

WALL ENTRANCE / SUDDEN CONTRACTION



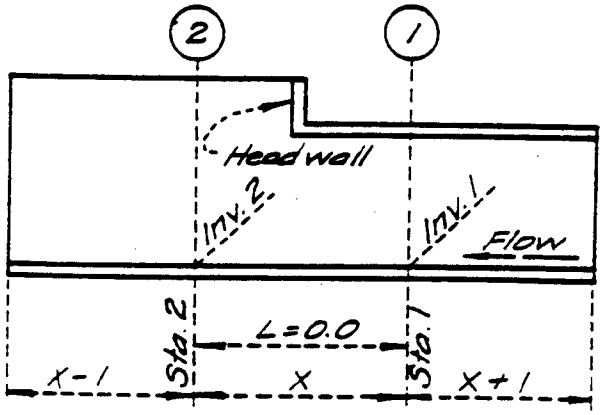
ELEVATION



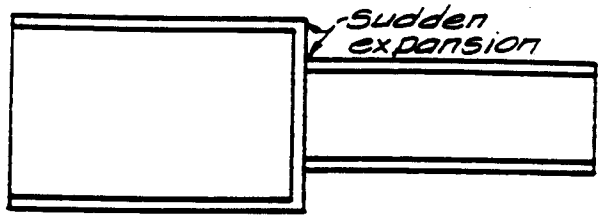
PLAN

FIG. 5-9

WALL EXIT / SUDDEN EXPANSION



ELEVATION



PLAN

FIG. 5-10

6.1 Channel and Conduit Section Description

Channels and conduits sections are classified as regular or irregular sections. The regular sections (Channel Types #1 - #4) are trapezoidal, rectangular channels, box conduits or pipes. The irregular sections (channel Types #5 and #6) can be natural river sections or irregular shaped improved sections with or without a cover. Piers or center walls can be included in any section except a pipe section.

6.1.1 Regular Channel Type Sections

The program utilizes the following regular sections:

Chan. Type # 1: Trapezoidal open top with or without piers. See Figure 6-1.

Chan. Type # 2: Rectangular open top with or without piers. See Figure 6-2.

Chan. Type # 3: Box, covered trapezoidal or covered rectangular with or without piers. See Figure 6-3.

Chan. Type # 4: Circular "pipe" one cell only. See Figure 6-4.

Note in multiple cell sections the cells may have variable width but must be of equal height and on the same invert elevation. The top elevation of all piers in both regular or irregular channels are assumed equal.

6.1.2 Irregular Sections

The program utilizes the following irregular cross sections:

Chan. Type # 5: Irregular open top with or without piers. See Figure 6-5.

Chan. Type # 6: Irregular covered top with or without piers. See figure 6-6.

6.1.3 Definitions & Restrictions for Irregular Sections

An irregular cross section (facing upstream) is defined by x and y coordinates of points i (x, y) given in a counter clockwise direction, from point i=1 to point i=n (minimum 3 points and maximum 99 points).

Point i = 1 (x, y) is where
x (1) = x min and if x (2) is also x minimum then
y (1) is greater than y (2)

LIMITATIONS

Location of x and y axis:

The center of the reference axis (x=o,y=o) must not fall on the perimeter of the cross section.

Flow Line:

A section can have only one low flow channel.

Section Shape:

A section is allowed one minimum and maximum in the x and y directions. For example between points from x minimum to x maximum the consecutive values of x must be equal or greater. From x maximum to x minimum the consecutive values of x must be equal or smaller. The same holds in the y direction.

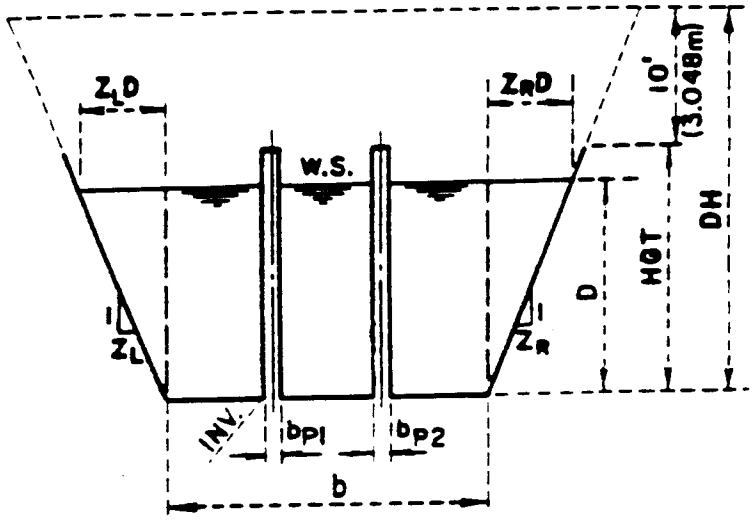
Piers:

The reference (x, y) axis for piers must be the same as used for the cross section. The "y" values are given from left to right.

REGULAR CHANNELS

FIG. 6-1

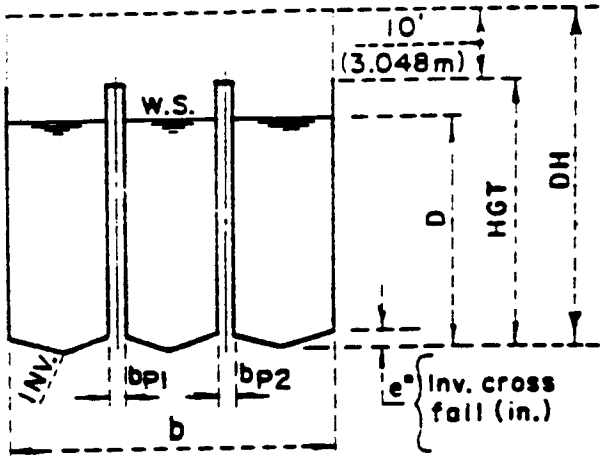
Trapezoidal section with or without piers.



CHAN TYPE 1

FIG. 6-2

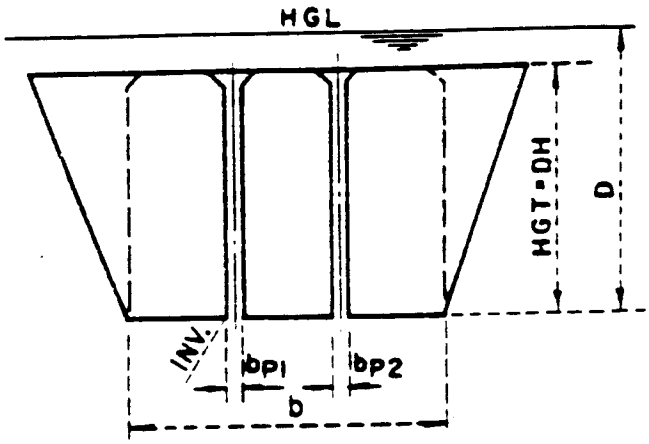
Rectangular section with or without piers.



CHAN TYPE 2

FIG. 6-3

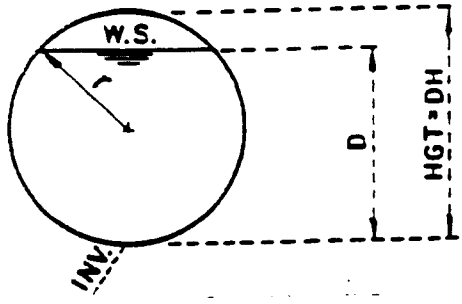
Box culvert, covered trap or rectangular section with or without piers.



CHAN TYPE 3

FIG. 6-4

Circular section (pipe) one cell only.



CHAN TYPE 4

IRREGULAR CHANNELS

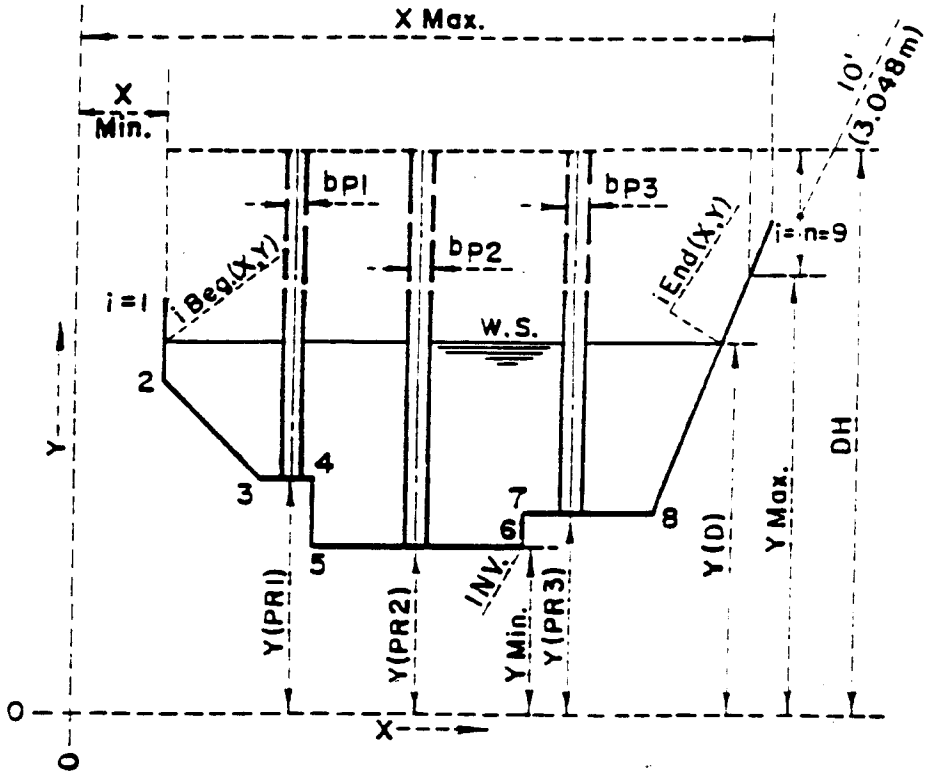


FIG. 6-5

Irregular open top section with or without piers.

CHAN TYPE 5

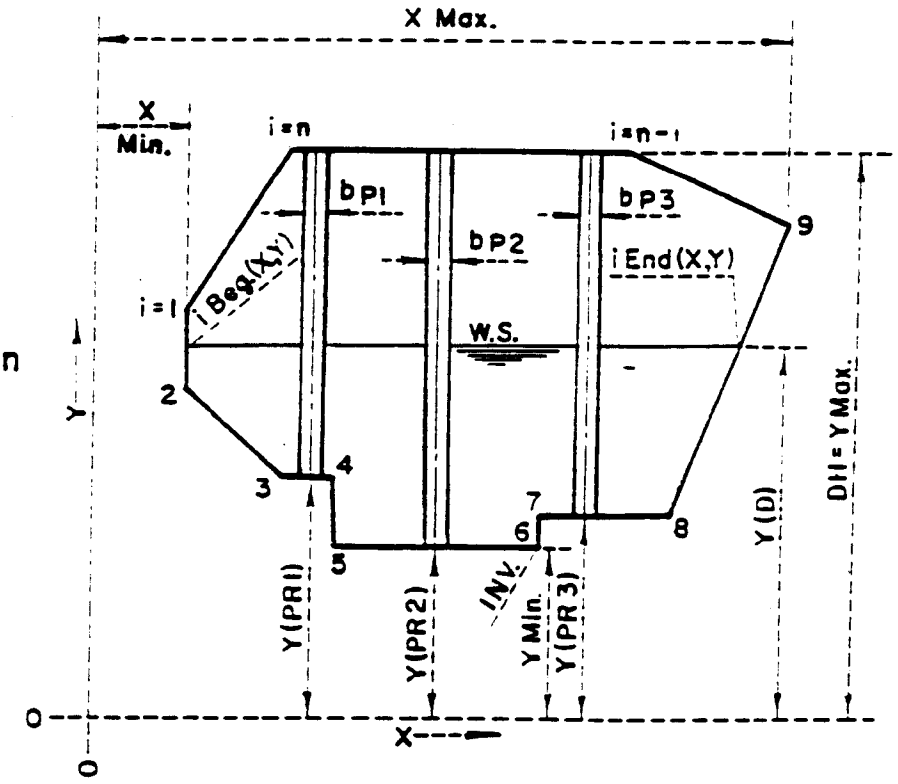


FIG. 6-6

Irregular covered section with or without piers.

CHAN TYPE 6

7.1 Input Data Description

This section contains the detailed description of each variable on each input card. With the exception of the three title cards all fields are fixed as indicated on the exhibits. If data is entered into a field the decimal points must be punched in the card column as shown.

Input data is placed by the user on Input Forms 1-4. All dimensions are in feet unless otherwise specified on the forms. Form 1 lists the title information and starting with the system outlet (SO) all elements are listed in chronological order upstream to the system headworks (SH). Form 2 is used to identify and list all data for regular cross sections, also to input data on piers for irregular cross sections. Form 3 is used to identify and list all irregular cross section data. Form 4 is used to list the starting Q at the system headworks. Form 4 is also used to list new starting Q's and optional Q factors when multiple water surface profile computation is desired.

7.1.1 TITLE CARDS

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

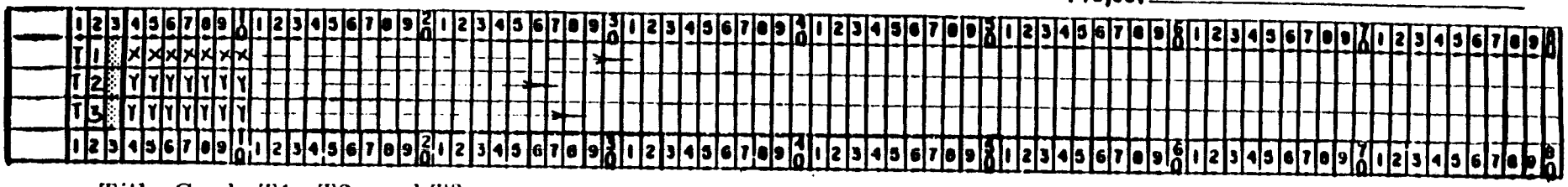
HYDRAULIC ANALYSIS COMPUTER PROGRAM

FO515 (FORM NO. 1)

Page_____ Of_____ Date_____

Name_____

Project_____



Title Cards T1, T2, and T3

Title cards are entered once per project
Title cards are used for output title

First title card T1 is required to start processing

Card Col.	Variable	Description
1 & 2	T1	Card identification characters
4 - 80	None	Number and alphabetical characters for output title

Second title card T2 is an optional card

Card Col.	Variable	Description
1 & 2	T2	Card identification characters
4 - 80	None	Number and alphabetical characters for output title

Third title card T3 is an optional card

Card Col.	Variable	Description
1 & 2	T3	Card identification characters
4 - 80	None	Number and alphabetical characters for output title

Legend

XX Required
YY Optional

7.1.2 ELEMENT CARDS

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

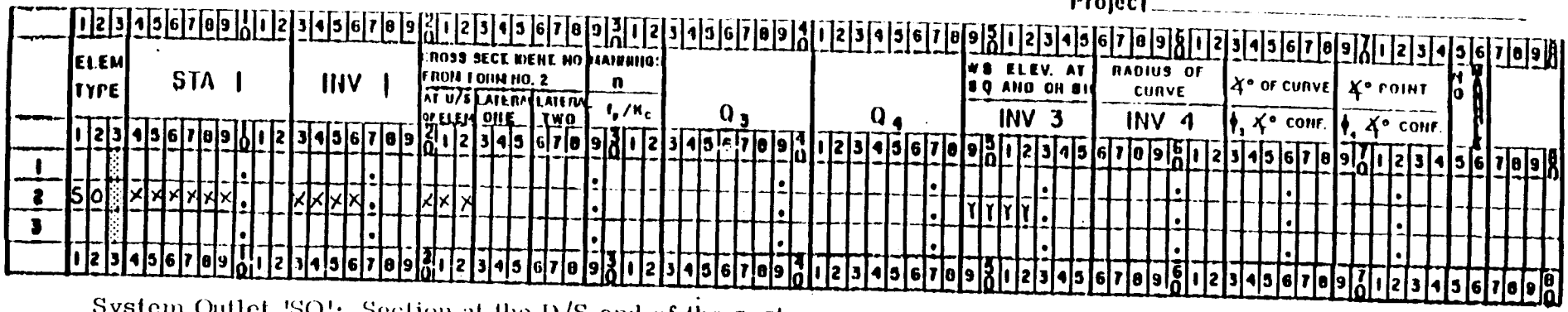
HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 1)

Page ____ Of ____ Date ____

Name ____

Project ____



System Outlet 'SO': Section at the D/S end of the system.

Card Col.	Variable	Description
1 & 2	SO	Card identification characters
4 thru 12	Sta. 1	A reference station from which the U/S end of all elements are measured
13 thru 19	Inv. 1	Invert elev. at the system outlet
20 thru 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify a specific section used at the U/S end of this element
49 thru 55	W.S. Elev. at SO	The W.S./HGL elevation control at the SO = 0.0 unless given otherwise. If the W.S./HGL elev. control is less than Inv. 1 + DC the program will set the control elevation equal to Inv. 1 + DC

Legend

XXX.XX Req'd Data
YYY.YY Optional Data

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

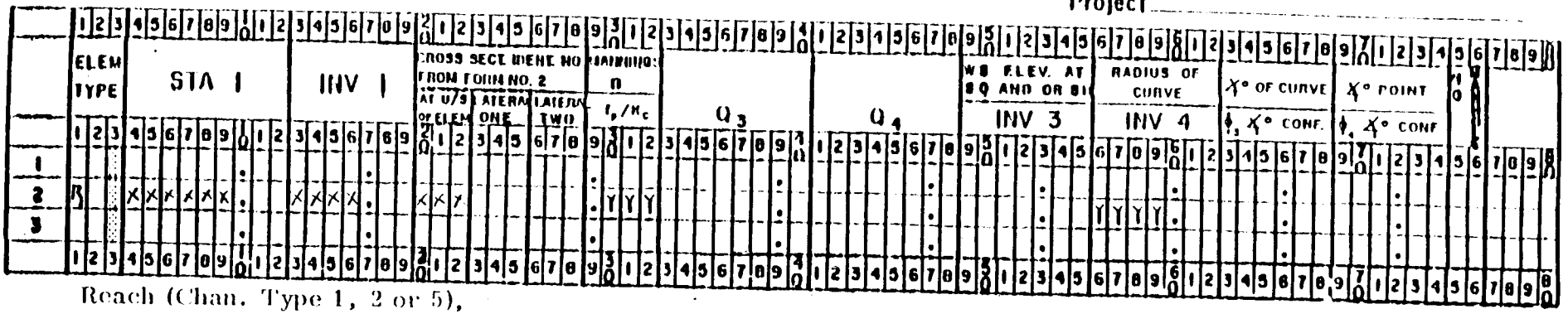
HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 1)

Page ____ Of ____ Date ____

Name ____

Project ____



Reach (Chan. Type 1, 2 or 5),

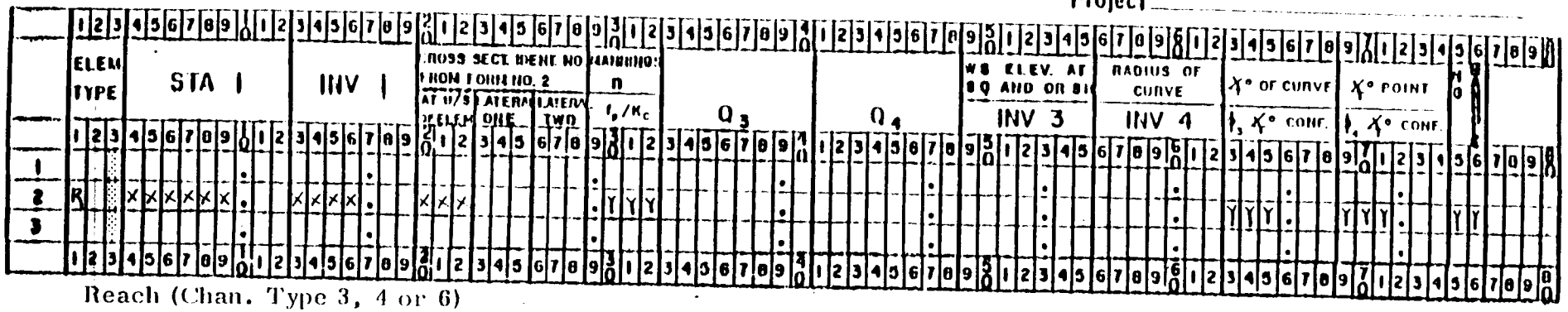
Card Col.	Variable	Description
1 or 2	R	Card identification character.
4 - 12	Sta. 1	Station at the U/S end.
13 - 19	Inv. 1	Invert elevation at the U/S end of this element.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify the specific section used at the U/S end of this element.
29 - 32	Manning's n	If left blank program uses .014.
56 - 62	Radius of curve	Radius of curvature necessary to compute super elev. (Beginning and end of curve must coincide with beginning and end of reach.)

Legend

XXXX.XX Required
YYYY.YY Optional

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
HYDRAULIC ANALYSIS COMPUTER PROGRAM
F0515 (FORM NO. 1)

Page ____ Of ____ Date ____
Name ____
Project ____



Reach (Chan. Type 3, 4 or 6)

Card Col.	Variable	Description
1 or 2	R	Card identification character.
4 - 12	STA 1	Station at the U/S end.
13 - 19	INV 1	Invert elevation at the U/S end.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to idenfity the specific section used at the U/S end.
29 - 32	Manning's n	If left blank program uses .014.
63 - 68	Angle of Curve	Angle of curve (0 to 180 degrees) necessary to compute bend loss. (Beginning and end of curve must coincide with beginning and end of reach.)
69 - 74	Angle Point	Angle point (0 to 15 degrees) necessary to compute angle point loss. (Location of angle pt. must coincide with beginning of the reach.)
75 - 76	No. of Manholes	Number of manholes (0 to 10) necessary to compute manhole losses.

Legend

XXXX. Req'd data
YYYY Optional data

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

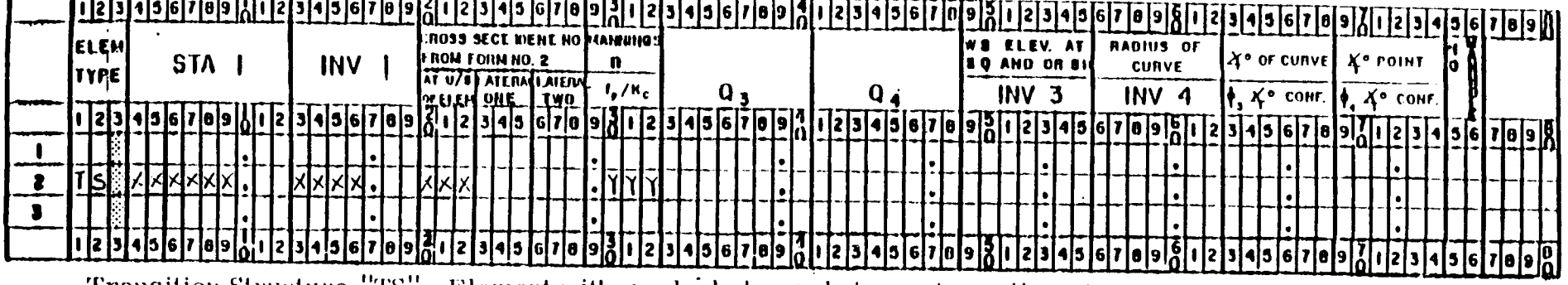
HYDRAULIC ANALYSIS COMPUTER PROGRAM

FO515 (FORM NO. 1)

Page_____ OF _____ Date_____

Name_____

Project_____



Transition Structure "TS": Element with gradual change between two adjacent sections. The length of transition must be greater than 0.0.

Card Col.	Variable	Description
1 & 2	TS	Req'd card identification characters.
4 - 12	STA 1	Station at the U/S end.
13 - 19	INV	Invert elevation at the U/S end.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify the specific section used at the U/S end.
29 - 32	Manning's n	If left blank program uses .014.

Legend

XXXX. Req'd data
YYYY Optional data

TS

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

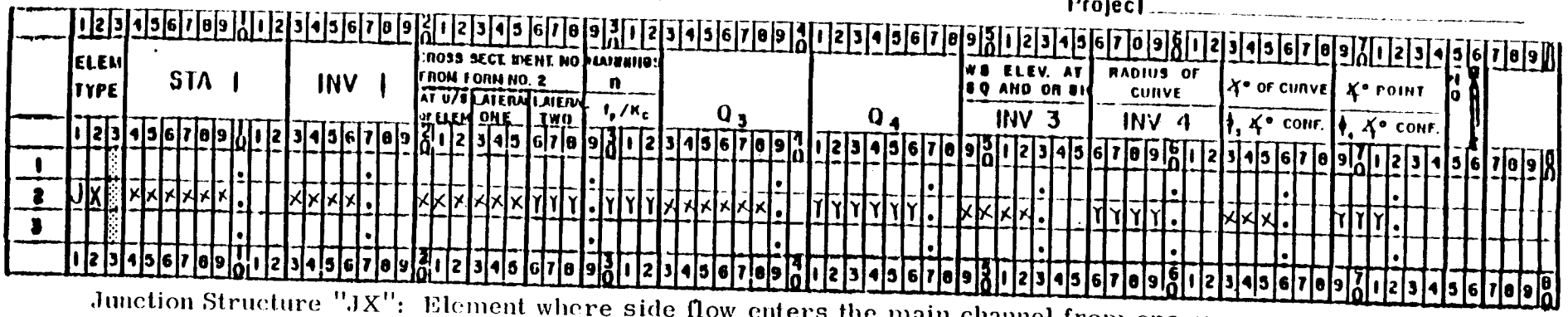
HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 1)

Page____ Of____ Date____

Name____

Project____



Junction Structure "JX": Element where side flow enters the main channel from one or two laterals. Length may be equal or greater than zero.

Card Col.	Variable	Description
1 & 2	JX	Req'd card identification characters.
4 - 12	STA 1	Station at the U/S end.
13 - 19	INV 1	Invert elevation at the U/S end.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify the specific section used at the U/S end.
23 - 25	Cross Sect. Ident. No. Lat. One	A number assigned on Form No. 2 to identify the specific section used for Lateral One.
26 - 28	Cross Sect. Ident. No. Lat. Two	A number assigned on Form No. 2 to identify the specific section used for Lateral Two.
29 - 32	Manning's n	If left blank program uses .014.
33 - 40	Q3	Flow rate in Lateral One.
41 - 48	Q4	Flow rate in Lateral Two.
49 - 55	INV 3	Invert elevation for Lateral One.
56 - 62	INV 4	Invert elevation for Lateral Two.
63 - 68	Θ 3	Confluence angle of Lateral One with main line (0.1 to 90 deg.)
69 - 74	Θ 4	Confluence angle of Lateral Two with main line (0.1 to 90 deg.)

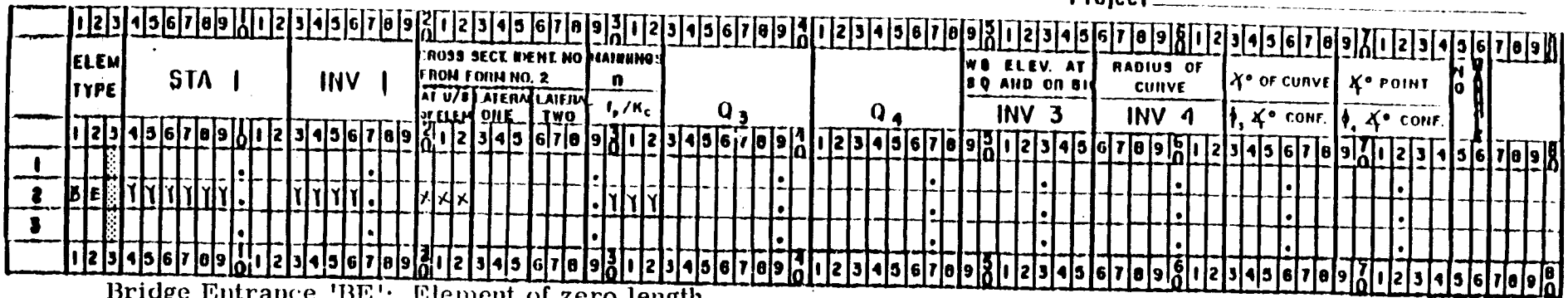
Legend

XXXX.XX Req'd data
YYYY.YY Optional data

JX

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
HYDRAULIC ANALYSIS COMPUTER PROGRAM
F0515 (FORM NO. 1)

Page ____ Of ____ Date ____
Name ____
Project ____



Bridge Entrance 'BE': Element of zero length
STA 1 = STA 2
INV 1 = INV 2
SECT 1 may be channel type 1, 2, 3, 5 and 6 and must not have piers
SECT 2 may be channel type 1, 2, 3, 5 and 6 and must have a minimum of one pier below the W.S. (No. of piers, 1 to 10)
(Sect 1 and Sect 2 must be identical except for the piers or cover)

Card Col.	Variable	Description
1 & 2	BE	Req'd card identification characters.
4 - 12	STA 1	Station at the U/S end. If Sta 1 is left blank it will be set equal to the sta. at the U/S end of the previous element.
13 - 19	INV 1	Invert Elevation at the U/S end. If INV 1 is left blank it will be set equal to the invert elevation at the U/S end of the previous element.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify the specific section used at the U/S end.
29 - 32	fp	A reduction factor for area of pier (if left blank program uses 1.0).

Legend

XXX.XX Req'd data
YYY.YY Optional data

BE

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

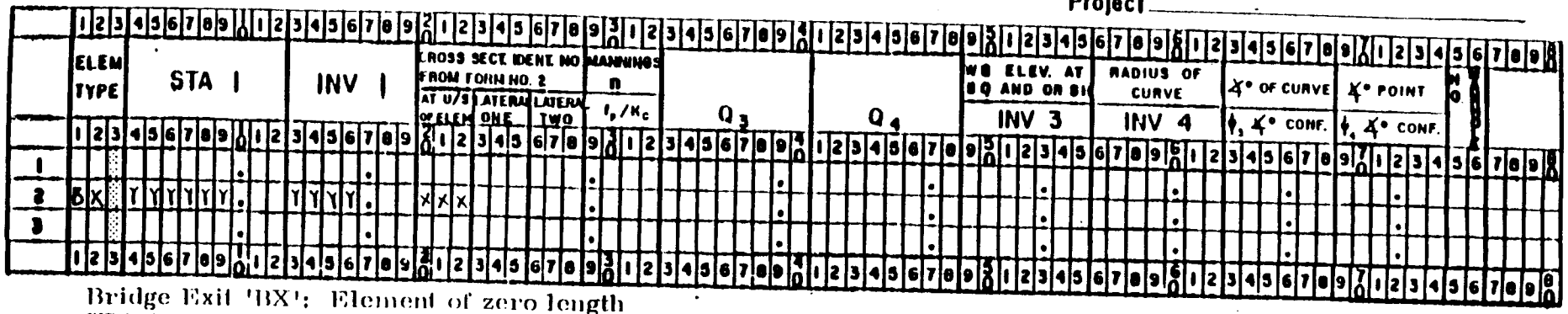
HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 1)

Page_____ Of_____ Date_____

Name_____

Project_____



Bridge Exit 'BX': Element of zero length
STA 1 = STA 2
INV 1 = INV 2
SECT 1 may be channel type No. 1, 2, 3, 5 or 6 and must have a minimum of one pier below the W.S. (No. piers, 1 to 10).
SECT 2 may be channel type No. 1, 2, 3, 5 or 6 and must not have piers.
(Sect 1 and Sect 2 must be identical except for the piers or covers)

Card Col.	Variable	Description
1 & 2	BX	Req'd card identification characters.
4 - 12	STA 1	Station at the U/S end. If STA 1 is left blank it will be set equal to the station at the U/S end of the previous element.
13 - 19	INV 1	Invert elevation at the U/S end. If INV 1 is left blank it will be set equal to the invert at the U/S end of the previous element.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify the specific section used at the U/S end.

Legend
XXXX. Req'd Data
YYYY Optional data

BX

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 1)

Page____ Of____ Date____

Name____

Project____

	ELEM TYPE	STA 1	INV 1	CROSS SECT IDENT NO FROM FORM NO. 2 AT U/S or ELEM	LATERAL ONE	LATERAL TWO	MANNING'S n / f_1/K_c	Q_3	Q_4	WS ELEV. AT SQ AND OR SI INV 3	RADIUS OF CURVE INV 4	$\Delta°$ OF CURVE $f_1, \Delta°$ CONF.	$\Delta°$ POINT $f_1, \Delta°$ CONF.	NO
1														
2	WE	Y Y Y Y Y .	Y Y Y .	X X X			. Y Y Y	.	.	.	.	.	.	
3														

WALL ENTRANCE (or Sudden Contraction) WE: Element of zero length

STA 1 = STA 2
INV 1 = INV 2
SECT. 1 Must be without piers.
SECT. 2 Sect. 2 may have piers (max. 10)

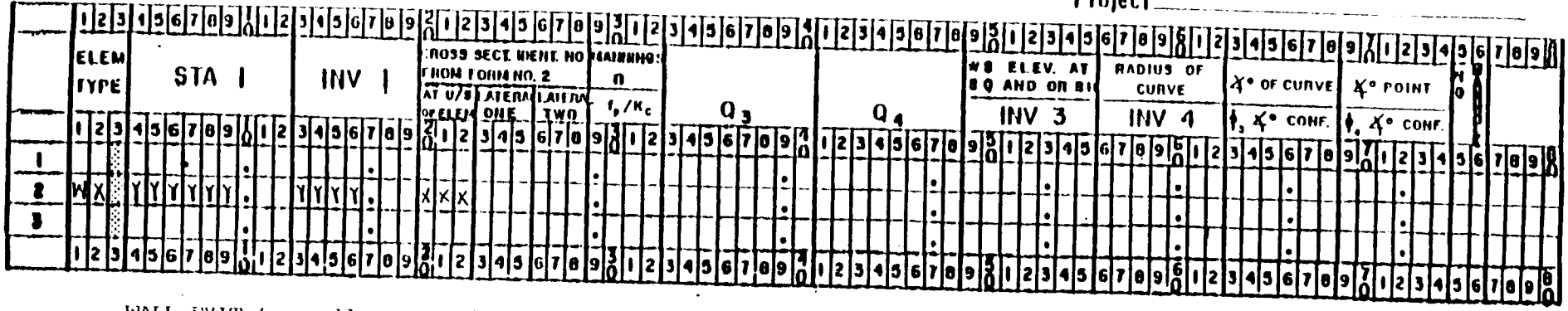
Card Col.	Variable	Description
1 & 2	WE	Required card identification Characters.
4 - 12	STA 1	Station at the u/s end. If Sta. 1 is left blank it will be set equal to Sta. 1 at the u/s end of the previous element.
13 - 19	INV 1	Invert elevation at the u/s end. If INV 1 is left blank it will be set equal to the invert elevation at the u/s end of the previous element.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify a specific section used at the u/s end.
29 - 32	Kc	A factor for computing the loss at WE (if left blank program uses 0.5).

legend XXXX.XX Required Data
YYYY.YY Optional Data

WE

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
HYDRAULIC ANALYSIS COMPUTER PROGRAM
F0515 (FORM NO. 1)

Page_____ Of _____ Date_____
Name_____
Project_____



WALL EXIT (or sudden expansion) WX is an element of a zero length

STA 1 = STA 2
INV 1 = INV 2
SECT. 1 may have piers (max. 10)
SECT. 2 must not have piers.

Card Col.	Variable	Description
1 & 2	WX	Required card identification characters.
4 - 12	STA 1	Station at the u/s end. If STA 1 is left blank it will be set equal to STA 1 at the u/s end of the previous element.
13 - 19	INV 1	Invert elevation at the u/s end. If INV 1 is left blank it will be set equal to the invert elevation at the u/s end of the previous element.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify a specific section used at the u/s end.

Legend XXX.XX Required Data
YYY.YY Optional Data

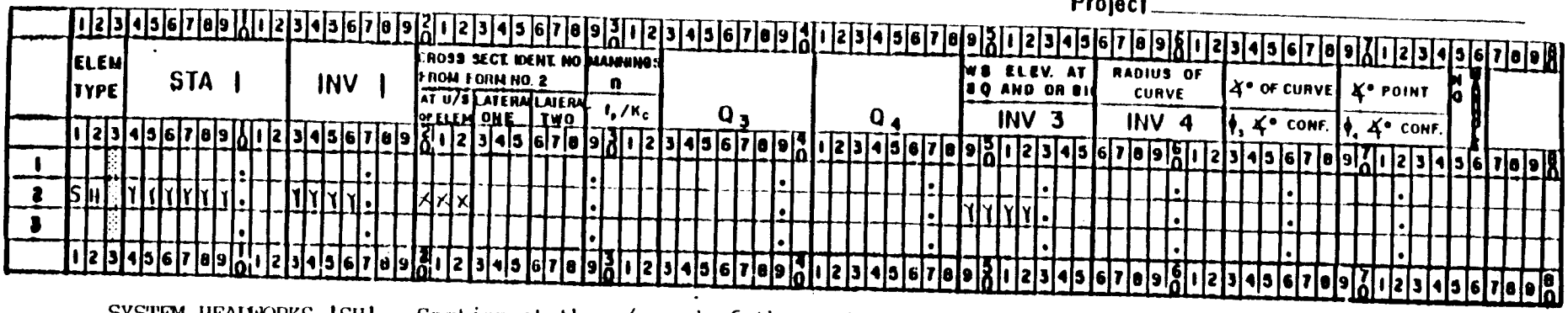
WX

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
HYDRAULIC ANALYSIS COMPUTER PROGRAM
F0515 (FORM NO. 1)

Page____ Of ____ Date____

Name____

Project____



SYSTEM HEADWORKS 'SH' Section at the u/s end of the system.

Card Col.	Variable	Description
1 & 2	SH	Required card identification characters.
4 - 12	STA 1	Station at the u/s end of the system. If Sta. 1 is left blank it will be set equal to the station at the u/s end of the previous element.
13 - 19	INV 1	Invert elevation at the u/s end of the system. If INV 1 is left blank it will be set equal to the invert elevation at the u/s end of the previous element.
20 - 22	Cross Sect. Ident. No.	A number assigned on Form No. 2 to identify a specific section used at the u/s end of the system.
49 - 55	W.S. Elev. at SH.	If left blank the program assumes critical depth control.

Legend XXX.XX Required Data
YYY.YY Optional Data

SH

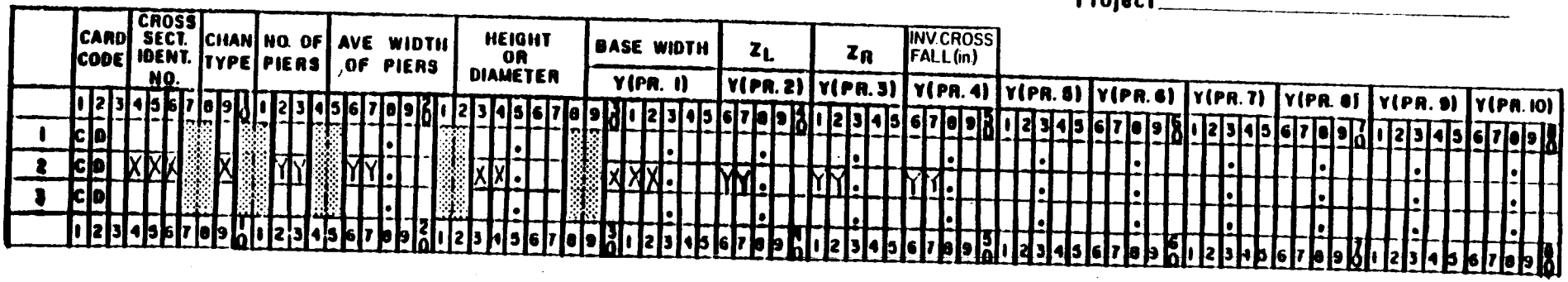
7.1.3 CD CARD CHANNEL DEFINITION

Name ____

Project ____

HYDRAULIC ANALYSIS COMPUTER PROGRAM

FO515 (FORM NO. 2)



CHANNEL CROSS SECTION DEFINITION CARDS "CD" (Regular)

Type 1 - Trapezoidal
Type 2 - Rectangular
Type 3 - Box or Covered Trapezoidal
Type 4 - Pipe

* Type-4 Pipe diameter.
** Type-4 Base width is not required for a pipe

Card Col.	Variable	Description
1 & 2	CD	Required card identification characters.
4 - 6	Cross Sect. Ident. No.	A number between 1 and 200 assigned to identify a section (a section need not be entered more than once per project).
9	Chan. Type	Channel Type number (see above).
12 - 13	No. of Piers	Number of piers (max. 10).
16 - 20	Ave Width of Piers	Sum of pier widths divided by number of piers.
23 - 27	Height *	Height of wall above the invert.
30 - 35	Base Width **	Width of base (see Figures 6-1 thru 6-3)
36 - 40	ZL	Left side slope.
41 - 45	ZR	Right side slope.
46 - 50	Invert Cross Fall	Invert slab cross fall in inches (see Figure 6-2) to be used with chan. type 2 (optional)

Legend XXX Required Data
YYY Optional Data

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

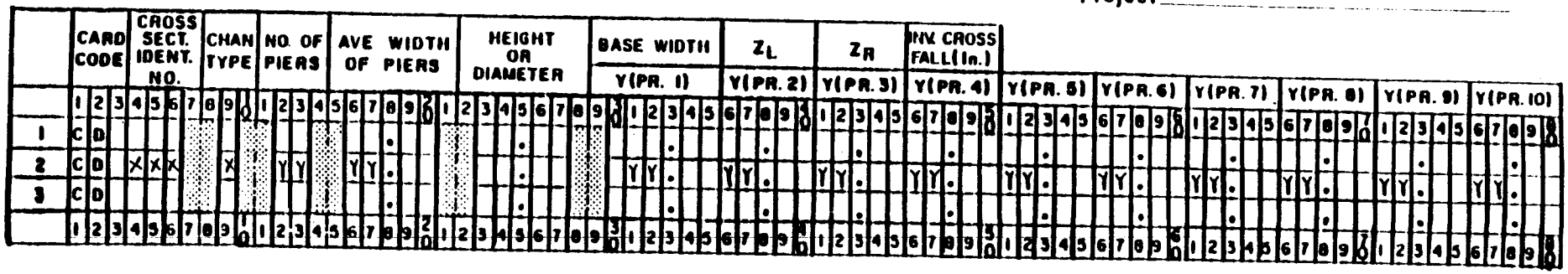
HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 2)

Page_____ Of_____ Date_____

Name_____

Project_____



CHANNEL CROSS SECTION DEFINITION CARD 'CD' (Irregular)

Type 5 - Irregular, open
Type 6 - Irregular, covered

NOTE: A points (PTS) card is required in conjunction with every CD card to complete the definition of an irregular cross section.

Card Col.	Variable	Description
1 & 2	CD	Required card identification characters.
4 - 6	Cross Sect. Ident. No.	A number between 1 and 200 assigned to identify a section. (A section need not be entered more than once per project).
9	Chan. Type	Channel type number (see above).
12 - 13	No. of Piers	Number of piers (max. 10).
16 - 20	Ave Width of Piers	Sum of pier widths divided by number of piers.
31 - 80	Y (PRi) from i=1 to i =10	The elevation of the bottom of the pier above the x-axis, (max Y(PR) = 99.99) All piers are assumed to have equal top elevation. For an irregular section with piers the X and Y axis must be the same for cross section and pier coordinates.

Legend XXX,XX required data
YYY,YY optional data

CD

7.1.4 PTS CARD "X & Y COORDINATES"

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

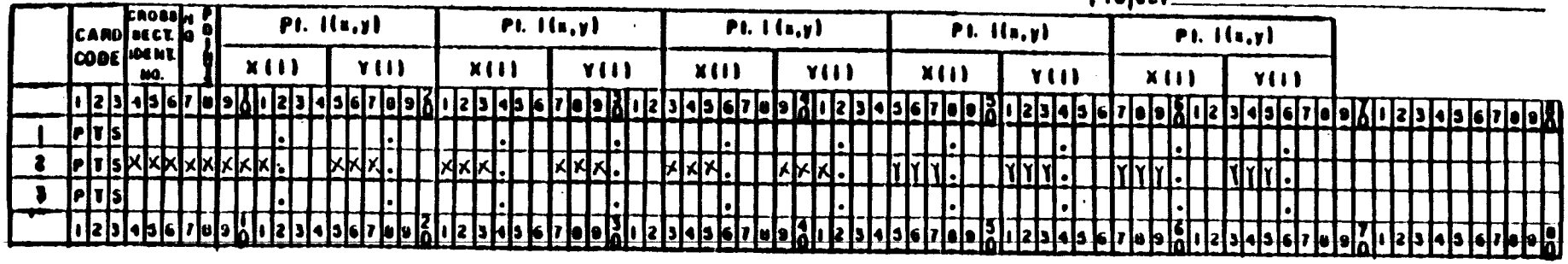
HYDRAULIC ANALYSIS COMPUTER PROGRAM

F0515 (FORM NO. 3)

Page ____ Of ____ Date ____

Name ____

Project ____



Irregular Channel Cross Section Points (X,Y) Card "PTS".

The PTS card is used in conjunction with the CD card to define irregular channel cross sections (Chan. Type No. 5 or No. 6).

Card Col.	Variable	Description
1 & 3	PTS	Required card identification characters.
4 - 6	Cross Sect. Ident. No.	The number assigned on the CD card.*
7 - 8	No. of Points	Specifies the number of points i (X,Y) in a cross section.*
9 - 68	Pt i(X,Y) x(i), Y(i)	The X and Y coordinates of the points in a cross section entered in consecutive order from left to right. (minimum 3 pts and maximum of 99 pts).

Legend XXX,XX Required Data
YYY,YY Optional Data

* If more than one line (card) is required to define a cross section the Ident. No. and the No. of pts. are entered on the first line (card).

PTS

7.1.5 Q CARDS

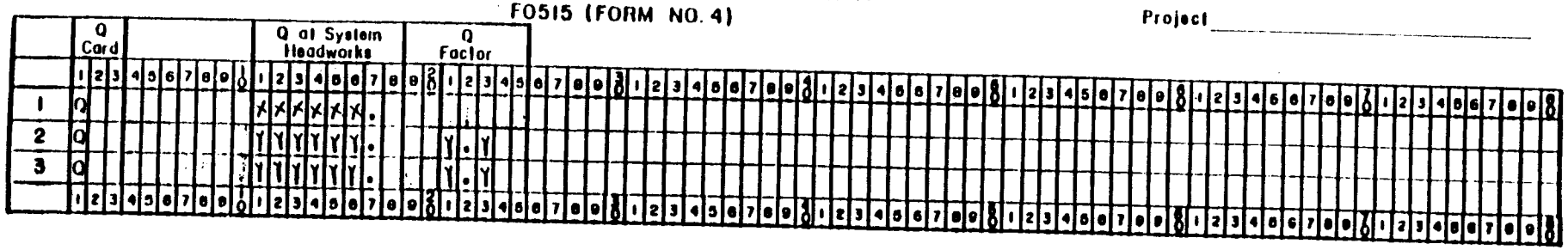
LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

Page ____ Of ____ Date ____

Name ____

Project ____

HYDRAULIC ANALYSIS COMPUTER PROGRAM
FO515 (FORM NO. 4)



Q CARDS

The first Q card is a required card to specify the Q at the headworks.

Card Col.	Variable	Description
1	Q	Required card identification character.
11 - 18	Q at S.H.	Q in cfs at the headworks.

Second, Third --- nth Q card

These are optional cards used to obtain w.s./hgl profiles for different flow rates in the system (see Sec. 3.3.3 for program limitation)

Card Col.	Variable	Description
1	Q	Required card identification character.
11 - 18	Q at S.H.	New Q at the system headworks.
21 - 25	Q factor	A multiplier which alters all lateral inflow in the system.

Legend XXX,XX Required Data
YYY,YY Optional Data

7.1.6 CONTROL CARDS

Control Cards

The District user need generally not be concerned with the control cards. However, if additional data on specific runs is required the following options are provided.

Control Card No. 1 - (trace switch card)
This is a required card that is placed ahead of the title cards. A "0" placed in CC 1 results in the regular composite water surface profile printout. (the District's Data processing center always includes this card with a 0 in CC1 unless otherwise instructed.) If "1" is placed in CC1 the element file will be included in the print out.

Control Card No. 2 - (JDEBUG)
This is a required card that is placed immediately before the Q CARDS. A 0 (zero) is placed in CC1 results in the regular composite water surface profile printout and plot. (The District's Data processing center always includes this card with a 0 (zero) in CC1 unless otherwise instructed) A "1" placed in CC1 results in the regular composite watersurface profile print out and plot and in addition prints out the lower and upper stage profiles and prints computational flow. A "2" placed in CC1 results in the regular composite water surface profile print out and plot and in addition prints out the lower and upper stage flow profiles.

7.2 SAMPLE INPUT

Los Angeles County
Flood Control District

DATA CENTER JOB WORK ORDER

DATE 2/27/79

DATE WANTED Same

DIVISION Design

PROGRAM No. F0515P

Job Requested

Punch	✓
Verify	✓
Reproduce	
Corrections	
Interpret	
Sort	
Merge	
Computer On-line	✓
Off-line Listing	

SPECIAL INSTRUCTIONS:

Last Card is a
9 card

Requested by

NAME Gaavin Pearson

PHONE 4088

ROOM 123-E

1788 FCD 1/79

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

HYDRAULIC ANALYSIS COMPUTER PROGRAM

FQ515 (FORM NO. 1)

 Date 2/27/79

Name GARVIN J. PEDERSON

Project No. 9202

T1 PSOJ NO 9202 UNIT 1, STA 3T+20 TO 7S+00

T2 SAMPLE PROBLEM BY GARVIN J. PEDERSON, FEB. 21, 1979

T3 TYPICAL BOX AND PIPE STORM DRAIN

	ELEM TYPE	STA 1	INV 1	CROSS SECT IDENT NO FROM FORM NO. 2 OR 3 AT D/S ELEM	LATERAL ONE	LATERAL TWO	MANNING'S n fp/Ke	Q_3	Q_4	WS ELEV. AT 60 AND OR 61 INV 3	RADIUS OF CURVE INV 4	ϕ_3 d°CONF	ϕ_4 d°CONF	NO. TUBES
1	SO	3+30.	7+40.45	1			.0			752.2				
2	R	4+00.5	7+41.4	1			.0 13					16.		
3	JX	4+16.5	7+41.46	1	2		.0 13	140.		741.6		30.		
4	R	4+30.	7+41.5	1			.0 15							1
5	R	4+70.	7+47.05	1			.0 13							
6	R	5+40.	7+51.7	1			.0 13							3
7	R	6+28.9	7+52.51	1			.0 13							3
8	JX	6+48.9	7+52.53	3	4		.0 13	85.		752.8		30.		
9	R	6+80.	7+58.7	3			.0 13							
10	R	7+20.	7+62.52	3			.0 13						4.5	
11	R	7+50.	7+62.63	3			.0 13							
12	SH			3										
13														
14														
15														
16														
17														
18														
19														
20														

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

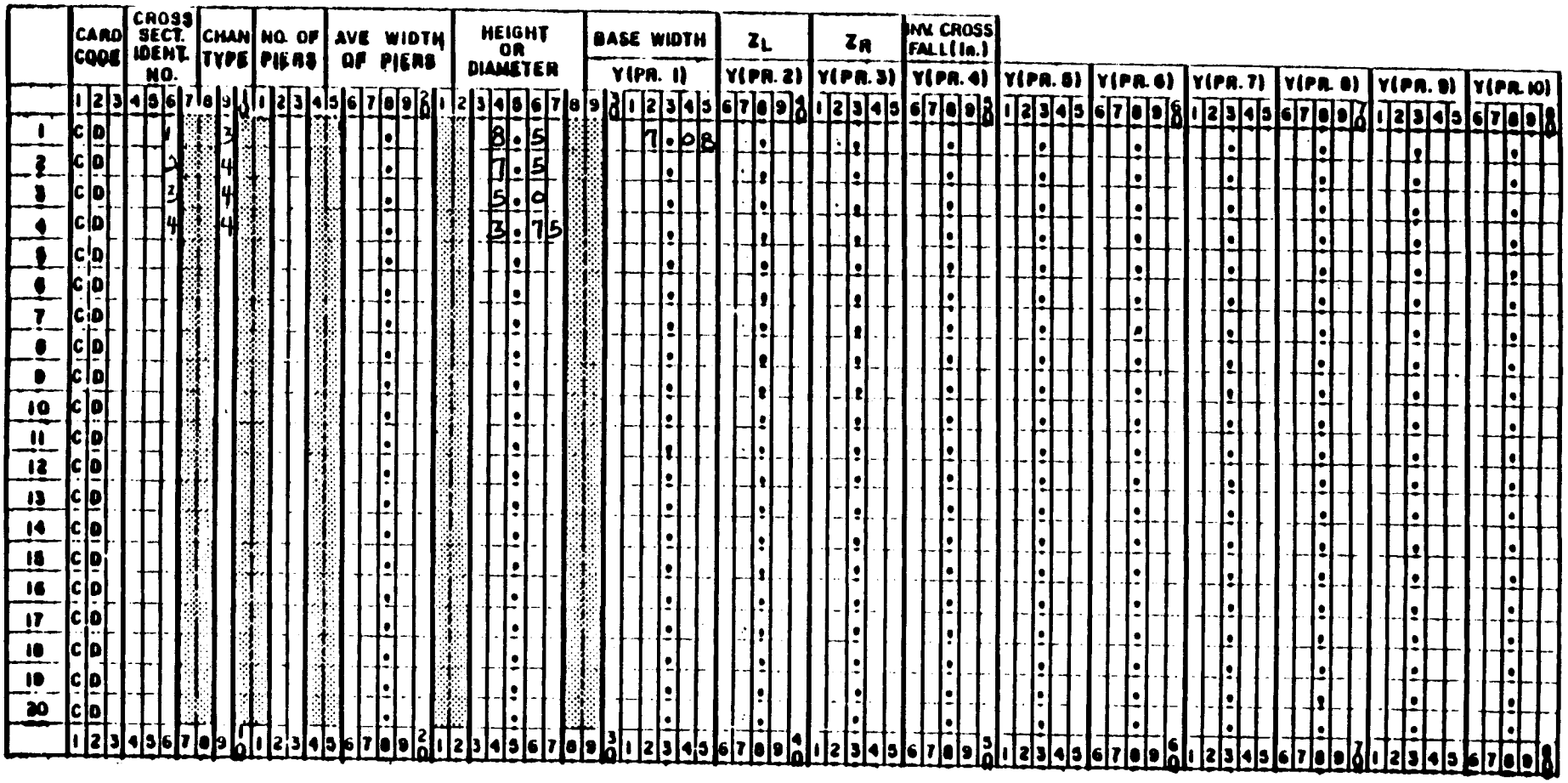
HYDRAULIC ANALYSIS COMPUTER PROGRAM

FQ515 (FORM NO. 2)

Page 2 of 3 Date 2/27/79

Name GARVIN J. PEDERSON

Project No. 9202



LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

HYDRAULIC ANALYSIS COMPUTER PROGRAM
F0515 (FORM NO. 4)

Page 3 of 3 Date 2/27/79
Name GARVIN J. PEDERSON
Project No. 9202

	Q Card			Q at System Headworks								Q Factor								
	1	2	3	1	2	3	4	5	6	7	8	9	10	1	2	3	4	5	6	7
1	Q						5	7	5	.					1					
2	Q									.					.					
3	Q									.					.					
4	Q									.					.					
5	Q									.					.					
6	Q									.					.					
7	Q									.					.					
8	Q									.					.					
9	Q									.					.					
10	Q									.					.					
11	Q									.					.					
12	Q									.					.					
13	Q									.					.					
14	Q									.					.					
15	Q									.					.					
16																				
17																				
18																				
19																				
20																				
21																				
22																				
23																				
24																				
25																				

Los Angeles County
Flood Control District

DATA CENTER JOB WORK ORDER

DATE 2/28/79

DATE WANTED Same

DIVISION Design

PROGRAM No. F0515P

Job Requested

Punch	✓
Verify	✓
Reproduce	
Corrections	
Interpret	
Sort	
Merge	
Computer On-line	✓
Off-line Listing	

SPECIAL INSTRUCTIONS:

Last Cago is a
9 cago

Requested By

NAME Saman J. Shahin

PHONE 4085

ROOM 123-F

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

HYDRAULIC ANALYSIS COMPUTER PROGRAM

FO515 (FORM NO. 1)

 Date 2/28/79

Name Saman J. Shahin

Project Ballona Creek

T1 BALLONA CREEK, STA 200+00 TO 255+00

T2 SAMPLE PROBLEM ENTERED BY SAMAN J. SHAHIN. DATE: FEB 28, 1979

T3 TYPICAL TRAPEZOIDAL CHANNEL WITH BRIDGE PIERS

	ELEM TYPE	STA 1	INV 1	CROSS SECT. IDENT. NO FROM FORM NO. 2 & 3 AT D/S ELEM	LATERAL ONE	LATERAL TWO	MANNING'S n fp/Ke	Q_3	Q_4	W.S. ELEV. AT 80 AND OR 81 INV 3	RADIUS OF CURVE INV 4	∠ OF CURVE ∅3 d°CONF	∠ POINT ∅4 d°CONF	NO
1	SO	20600.0	9.54	1										
2	R	20800.0	9.94	1										
3	R	21300.0	10.95	1										
4	BX			2										
5	R	21380.0	11.11	2										
6	BE						.67							
7	R	22319.76	13.00											
8	R	22419.76	15.00											
9	R	22430.68	15.11											
10	BX			3										
11	R	22560.68	15.37	3										
12	BE						.67							
13	R	25500.00	23.08											
14	SI													
15														
16														
17														
18														
19														
20														

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

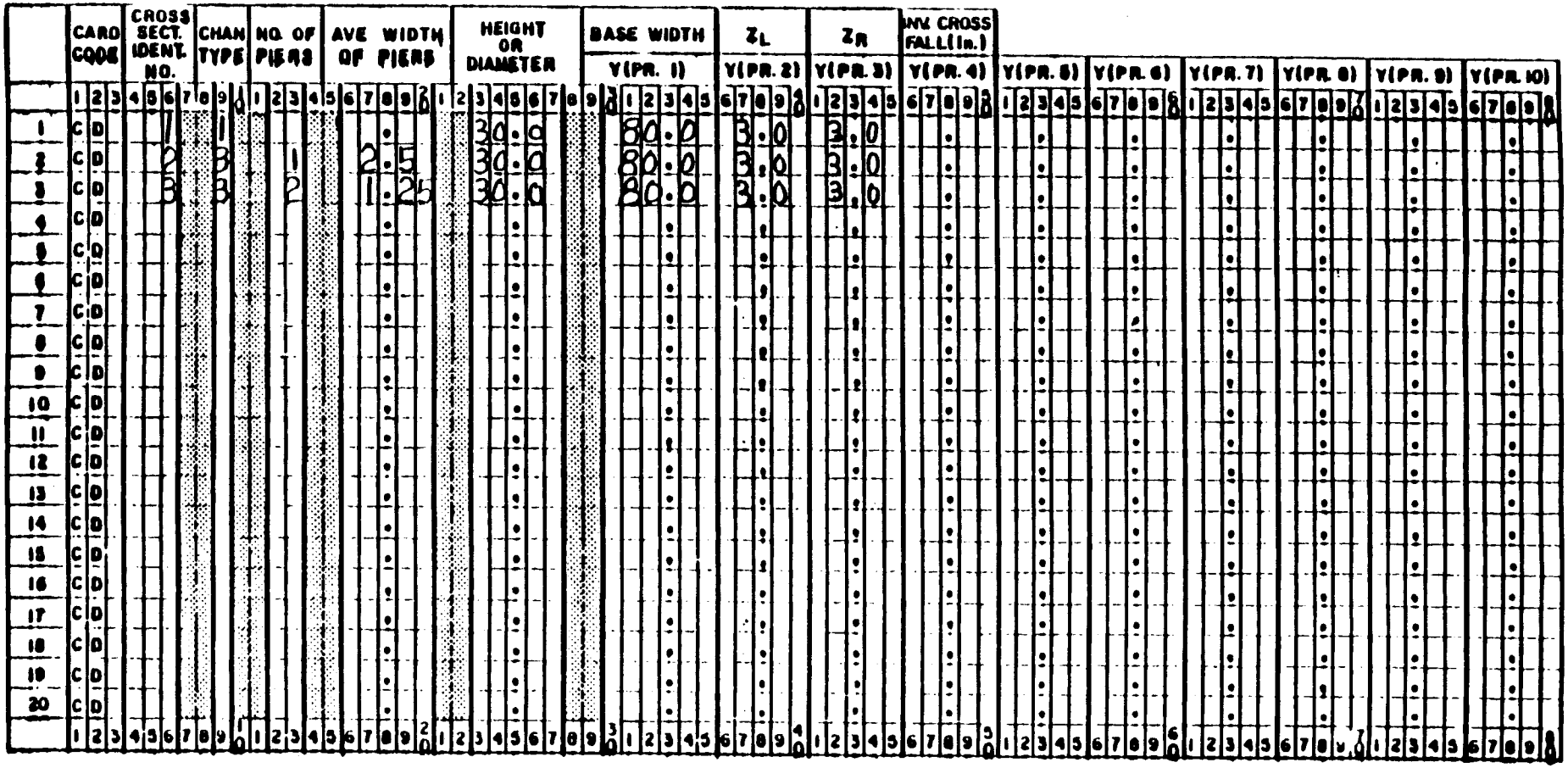
HYDRAULIC ANALYSIS COMPUTER PROGRAM

FO515 (FORM NO. 2)

Page 2 of 3 Date 2/28/79

Name Saman J. Shahin

Project Ballona Creek



LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

HYDRAULIC ANALYSIS COMPUTER PROGRAM
F0515 (FORM NO. 4)

Page 3 of 3 Date 2/28/79
Name Saman J. Shahin
Project Ballona Creek

	Q Card				Q at System Headworks								Q Factor								
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21
1	Q								5	8	5	0	0	.	0						
2	Q																				
3	Q																				
4	Q																				
5	Q																				
6	Q																				
7	Q																				
8	Q																				
9	Q																				
10	Q																				
11	Q																				
12	Q																				
13	Q																				
14	Q																				
15	Q																				
16																					
17																					
18																					
19																					
20																					
21																					
22																					
23																					
24																					
25																					
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21

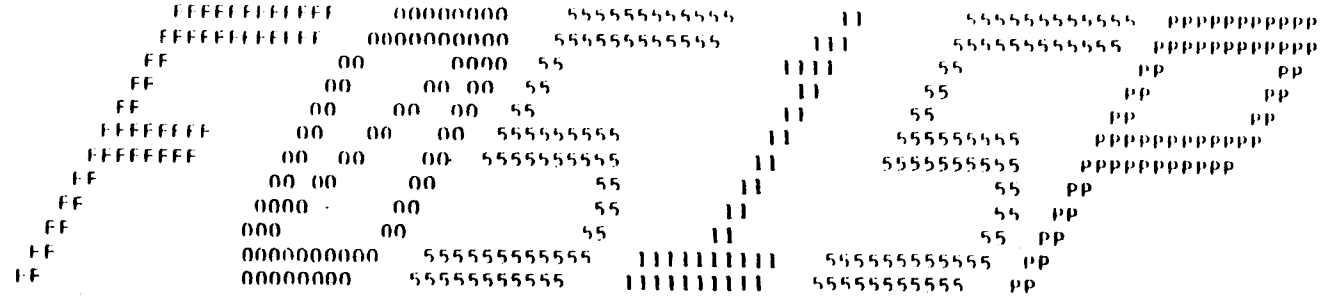
8.1 OUTPUT DATA DESCRIPTION

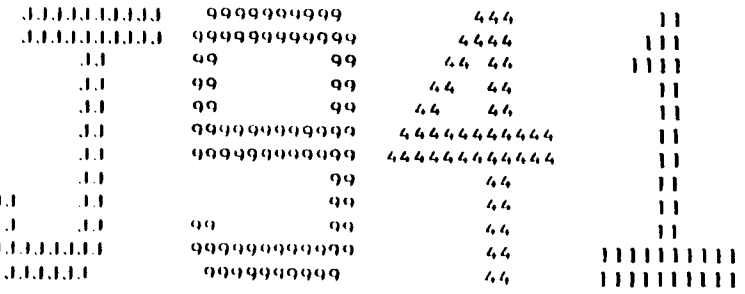
8.1.1 Normal Output

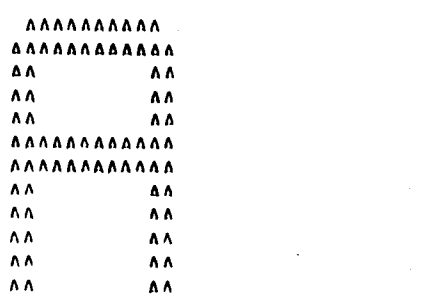
a) Listing of channel cross section definitions (CD card data)
b) Listing irregular channel cross section points (PTS data)
c) Listing of element input data
d) Listing of title data
e) Listing of computed water surface profile or pressure gradient data
f) Plotting of the conduit and water surface data
g) If additional Q's cards are inputed then multiple output will occur for each set of Q's

8.1.2 Optional Output

Program F0515P will provide, in addition to the above output, a listing of all lower stage water surface profile points and all upper stage water surface profile points.








```
****A  END  JOB  941  F05ISP  BIN42
****A  END  JOB  941  F05ISP  BIN42
****A  END  JOB  941  F05ISP  BIN42
****A  END  JOB  941  F05ISP  BIN42
****A  END  JOB  941  F05ISP  BIN42
****A  END  JOB  941  F05ISP  BIN42
```

```
ROOM A108   2.09.13 PM 08 MAR 79  R4.PR1   SYS 1558  JOB  941  END  A****
ROOM A108   2.09.13 PM 08 MAR 79  R4.PR1   SYS 1558  JOB  941  END  A****
ROOM A108   2.09.13 PM 08 MAR 79  R4.PR1   SYS 1558  JOB  941  END  A****
ROOM A108   2.09.13 PM 08 MAR 79  R4.PR1   SYS 1558  JOB  941  END  A****
ROOM A108   2.09.13 PM 08 MAR 79  R4.PR1   SYS 1558  JOB  941  END  A****
ROOM A108   2.09.13 PM 08 MAR 79  R4.PR1   SYS 1558  JOB  941  END  A****
```

CARD CODE	SECT NO	CHN TYPE	NO OF PIERS	AVE PIER WIDTH	HEIGHT 1 DIAMETER	BASE WIDTH	ZL	ZR	INV DROP	Y(1)	Y(2)	Y(3)	Y(4)	Y(5)	Y(6)	Y(7)	Y(8)	Y(9)	Y(10)
CD	1	3	0	0.0	8.50	7.08	0.0	0.0	0.0										
CD	2	4			5.00														
CD	3	4			7.50														
CD	4	4			3.75														

WATER SURFACE PROFILE - TITLE CARD LISTING

WATER SURFACE PROFILE - TITLE CARD LISTING

HEADING LINE NO 1 IS -

PROJ NO 9202 UNIT 1, STA 372+20 TO 75+00

HEADING LINE NO 2 IS -

SAMPLE PROBLEM ENTERED BY GARVIN J. PEDERSON DATE : FEB 27 , 1979

HEADING LINE NO 3 IS -

TYPICAL BOX AND PIPE STORM DRAIN.

WATER SURFACE PROFILE - TITLE CARD LISTING

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO 1 IS A SYSTEM OUTLET

U/S DATA	STATION	INVERT	SECT	W S ELEV
	3720.00	740.48	1	752.20

ELEMENT NO 2 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	4000.50	741.40	1	0.013	0.0	16.00	0.0	0

ELEMENT NO 3 IS A JUNCTION

U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	03	04	INVERT-3	INVERT-4	PHI 3	PHI 4
	4016.50	741.46	1	2	0	0.013	140.0	0.0	741.60	0.0	30.00	0.0

ELEMENT NO 4 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	4030.00	741.50	1	0.013	0.0	0.0	0.0	1

ELEMENT NO 5 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	4170.00	747.05	1	0.013	0.0	0.0	0.0	0

ELEMENT NO 6 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	5240.00	751.70	1	0.013	0.0	0.0	0.0	3

ELEMENT NO 7 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	6628.90	752.51	1	0.013	0.0	0.0	0.0	3

ELEMENT NO 8 IS A JUNCTION

U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	03	04	INVERT-3	INVERT-4	PHI 3	PHI 4
	6648.90	752.53	3	4	0	0.013	85.0	0.0	752.80	0.0	30.00	0.0

ELEMENT NO 9 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	6800.00	758.70	3	0.013	0.0	0.0	0.0	0

ELEMENT NO 10 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	7490.00	762.52	3	0.013	0.0	0.0	4.50	0

ELEMENT NO 11 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	7500.00	762.53	3	0.013	0.0	0.0	0.0	0

ELEMENT NO 12 IS A SYSTEM HEADWORKS

U/S DATA	STATION	INVERT	SECT	W S ELEV
	7500.00	762.53	3	0.0

WSPSP

WATER SURFACE PROFILE - TITLE CARD LISTING

WATER SURFACE PROFILE - ELEMENT CARD LISTING

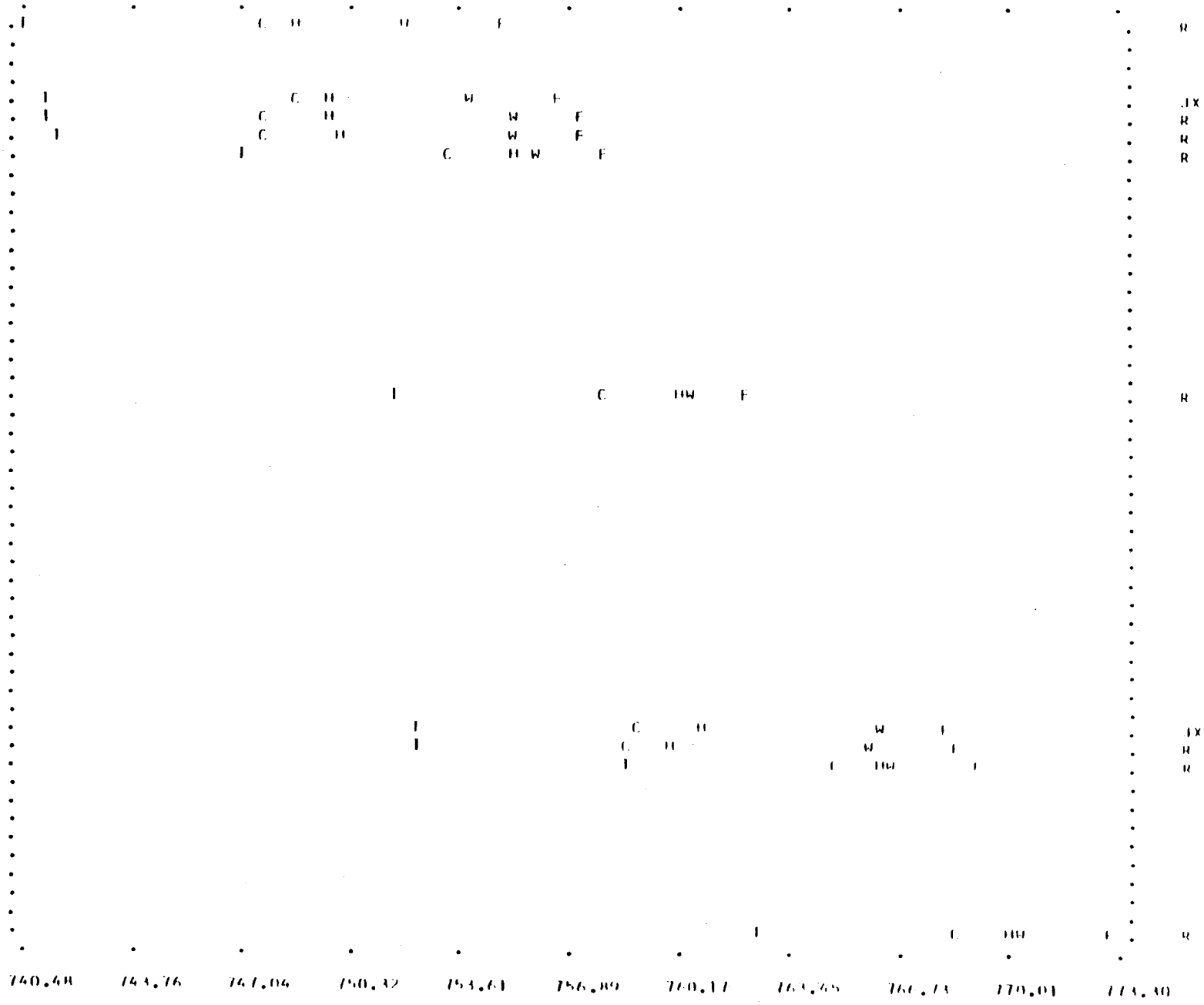
NO EDIT ERRORS ENCOUNTERED-COMPUTATION IS NOW BEGINNING

** WARNING NO. 2 ** - WATER SURFACE ELEVATION GIVEN IS LESS THAN OR EQUALS INVERT ELEVATION IN HOWKOS. W.S.ELEV = INV + DC

EQ515P
WATER SURFACE PROFILE TESTING
PROJ NO 9202 UNIT 1, STA 372+20 TO 75+00
SAMPLE PROBLEM ENTERED BY GARVIN J. PEDERSON DATE : FEB 27 , 1979
TYPICAL BOX AND PIPE STORM DRAIN.

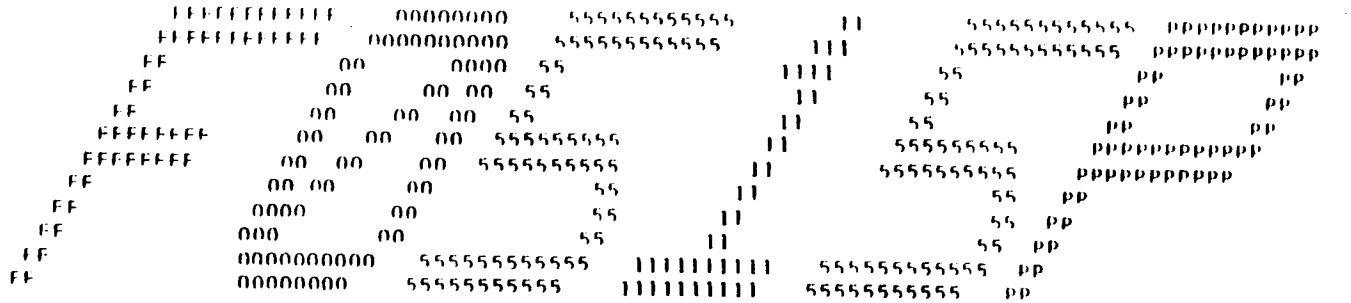
STATION	INVERT ELEV	DEPTH OF FLOW	W.S. ELEV	Q	VEL	VEL HEAD	ENERGY GRD.EL.	SUPER ELEV	CRITICAL DEPTH		HGT/ DIA	BASE/ ID NO.	ZL	NO PIER	AVNPR
L/ELEM	SO					SF AVE	HF			NORM DEPTH			ZR		
3720.00	740.48	11.720	752.200	800.0	13.35	2.767	754.967	0.0	7.349		8.50	7.08	0.0	0	0.0
280.50	0.00328					.005702	1.60			8.500			0.0		
4000.50	741.40	12.633	754.033	800.0	13.35	2.767	756.800	0.0	7.349		8.50	7.08	0.0	0	0.0
JUNCT STR	0.00375					.004791	0.08						0.0		
4016.50	741.46	13.969	755.429	660.0	11.01	1.883	757.312	0.0	6.464		8.50	7.08	0.0	0	0.0
13.50	0.00296					.003881	0.05			8.195			0.0		
4030.00	741.50	14.075	755.575	660.0	11.01	1.883	757.458	0.0	6.464		8.50	7.08	0.0	0	0.0
140.00	0.03964					.003881	0.54			2.974			0.0		
4170.00	747.05	9.069	756.119	660.0	11.01	1.883	758.002	0.0	6.464		8.50	7.08	0.0	0	0.0
1070.00	0.00435					.003881	4.15			6.997			0.0		
5240.00	751.70	8.854	760.554	660.0	11.01	1.883	762.437	0.0	6.464		8.50	7.08	0.0	0	0.0
1388.90	0.00058					.003881	5.39			8.500			0.0		
6628.90	752.51	13.716	766.226	660.0	11.01	1.883	768.109	0.0	6.464		8.50	7.08	0.0	0	0.0
JUNCT STR	0.00100					.004744	0.09						0.0		
6668.90	752.53	13.324	765.856	575.0	13.02	2.630	768.486	0.0	6.172		7.50	0.0	0.0	0	0.0
151.10	0.04083					.005607	0.85			3.161			0.0		
6800.00	758.70	8.002	766.702	575.0	13.02	2.630	769.332	0.0	6.172		7.50	0.0	0.0	0	0.0
690.00	0.00554					.005607	3.87			6.192			0.0		
7490.00	762.52	8.090	770.610	575.0	13.02	2.630	773.240	0.0	6.172		7.50	0.0	0.0	0	0.0
10.00	0.00100					.005607	0.06			7.500			0.0		
7500.00	762.53	8.136	770.666	575.0	13.02	2.630	773.296	0.0	6.172		7.50	0.0	0.0	0	0.0

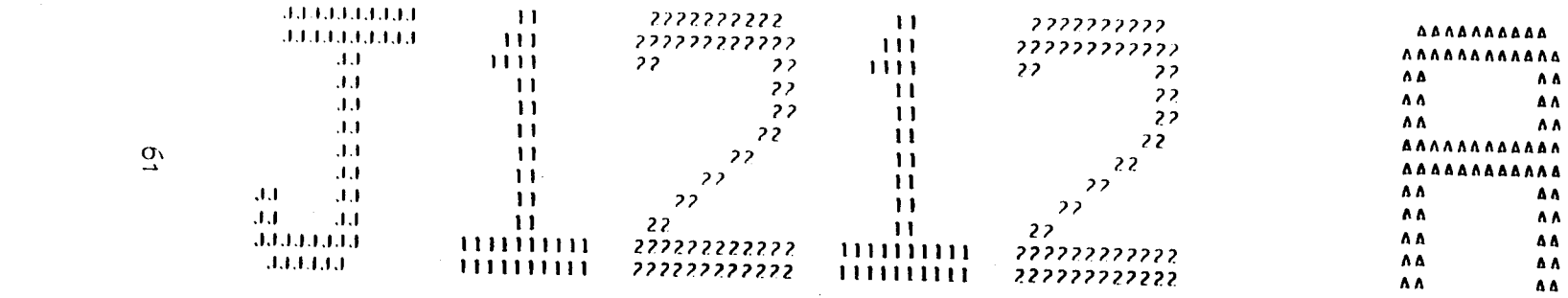
PROJ NO 9202 UNIT 1, STA 372+20 TO 75+00
SAMPLE PROBLEM ENTERED BY GARVIN J. PEDERSON DATE : FEB 27 , 1979
TYPICAL BOX AND PIPE STORM DRAIN.



NOTES

1. GLOSSARY
   I = INVERT ELEVATION
   C = CRITICAL DEPTH
   W = WATER SURFACE ELEVATION
   H = HEIGHT OF CHANNEL
   E = ENERGY GRADE LINE
   X = CURVES CROSSING OVER
   B = BRIDGE ENTRANCE OR EXIT
   Y = WALL ENTRANCE OR EXIT
2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY






```
****A  START  JOB 1212  E0515P    RIN42
****A  START  JOB 1212  E0515P    RIN42
****A  START  JOB 1212  E0515P    RIN42
****A  START  JOB 1212  E0515P    RIN42
****A  START  JOB 1212  E0515P    RIN42
****A  START  JOB 1212  E0515P    RIN42

ROOM A108    7.59.05 AM 28 FEB 79   R4.PR1   SYS 155A  JOB 1212  START  A****
ROOM A108    7.59.05 AM 28 FEB 79   R4.PR1   SYS 155A  JOB 1212  START  A****
ROOM A108    7.59.05 AM 28 FEB 79   R4.PR1   SYS 155A  JOB 1212  START  A****
ROOM A108    7.59.05 AM 28 FEB 79   R4.PR1   SYS 155A  JOB 1212  START  A****
ROOM A108    7.59.05 AM 28 FEB 79   R4.PR1   SYS 155A  JOB 1212  START  A****
ROOM A108    7.59.05 AM 28 FEB 79   R4.PR1   SYS 155A  JOB 1212  START  A****
```

EQSISP

WATER SURFACE PROFILE - CHANNEL DEFINITION LISTING PAGE 1

CARD CODE	SECT NO	CHN TYPE	NO OF PIERS	AVE PIER WIDTH	HEIGHT 1 DIAMETER	BASE WIDTH	Z1	Z2	INV DROP	Y(1)	Y(2)	Y(3)	Y(4)	Y(5)	Y(6)	Y(7)	Y(8)	Y(9)	Y(10)
CD	1	1	0	0.0	30.00	80.00	3.00	3.00	0.0										
CD	2	3	1	2.50	30.00	80.00	3.00	3.00	0.0										
CD	3	3	2	1.25	30.00	80.00	3.00	3.00	0.0										

WATER SURFACE PROFILE - TITLE CARD LISTING

WATER SURFACE PROFILE - TITLE CARD LISTING

HEADING LINE NO 1 IS -

BALLONA CREEK , STA 206+00 TO 255+00

HEADING LINE NO 2 IS -

SAMPLE PROBLEM ENTERED BY SAMAN J. SHAHIN DATE : FEB 27 , 1979

HEADING LINE NO 3 IS -

TYPICAL TRAPEZOIDAL CHANNEL WITH PIERS .

WATER SURFACE PROFILE - TITLE CARD LISTING

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO 1 IS A SYSTEM OUTLET

U/S DATA	STATION	INVERT	SECT	W S ELEV
	20600.00	9.54	1	0.0

ELEMENT NO 2 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	21300.00	10.95	1	0.014	0.0	0.0	0.0	0

ELEMENT NO 3 IS A BRIDGE EXIT

U/S DATA	STATION	INVERT	SECT
	21300.00	10.95	2

ELEMENT NO 4 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	21380.00	11.11	2	0.014	0.0	0.0	0.0	0

ELEMENT NO 5 IS A BRIDGE ENTRANCE

U/S DATA	STATION	INVERT	SECT	EP
	21380.00	11.11	1	0.670

ELEMENT NO 6 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	22319.76	13.00	1	0.014	0.0	0.0	0.0	0

ELEMENT NO 7 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	22419.76	15.00	1	0.014	0.0	0.0	0.0	0

ELEMENT NO 8 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	22460.68	15.11	1	0.014	0.0	0.0	0.0	0

ELEMENT NO 9 IS A BRIDGE EXIT

U/S DATA	STATION	INVERT	SECT
	22460.68	15.11	3

ELEMENT NO 10 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	22560.68	15.37	3	0.014	0.0	0.0	0.0	0

ELEMENT NO 11 IS A BRIDGE ENTRANCE

U/S DATA	STATION	INVERT	SECT	EP
	22560.68	15.37	1	0.670

ELEMENT NO 12 IS A REACH

U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
	25500.00	23.08	1	0.014	0.0	0.0	0.0	0

ELEMENT NO 13 IS A SYSTEM HEADWORKS

U/S DATA	STATION	INVERT	SECT	W S ELEV
	25500.00	23.08	1	0.0

NO EDIT ERRORS ENCOUNTERED-COMPUTATION IS NOW BEGINNING

** WARNING NO. 2 ** - WATER SURFACE ELEVATION GIVEN IS LESS THAN OR EQUALS INVERT ELEVATION IN HOOKDS. W.S.ELEV = INV + DC

FOSTSP
WATER SURFACE PROFILE LISTING
BALLONA CREEK , STA 20+00 TO 255+00
SAMPLE PROBLEM ENTERED BY SAMAN J. SHAHIN DATE : FEB 27 , 1979
TYPICAL TRAPEZOIDAL CHANNEL WITH PIERS .

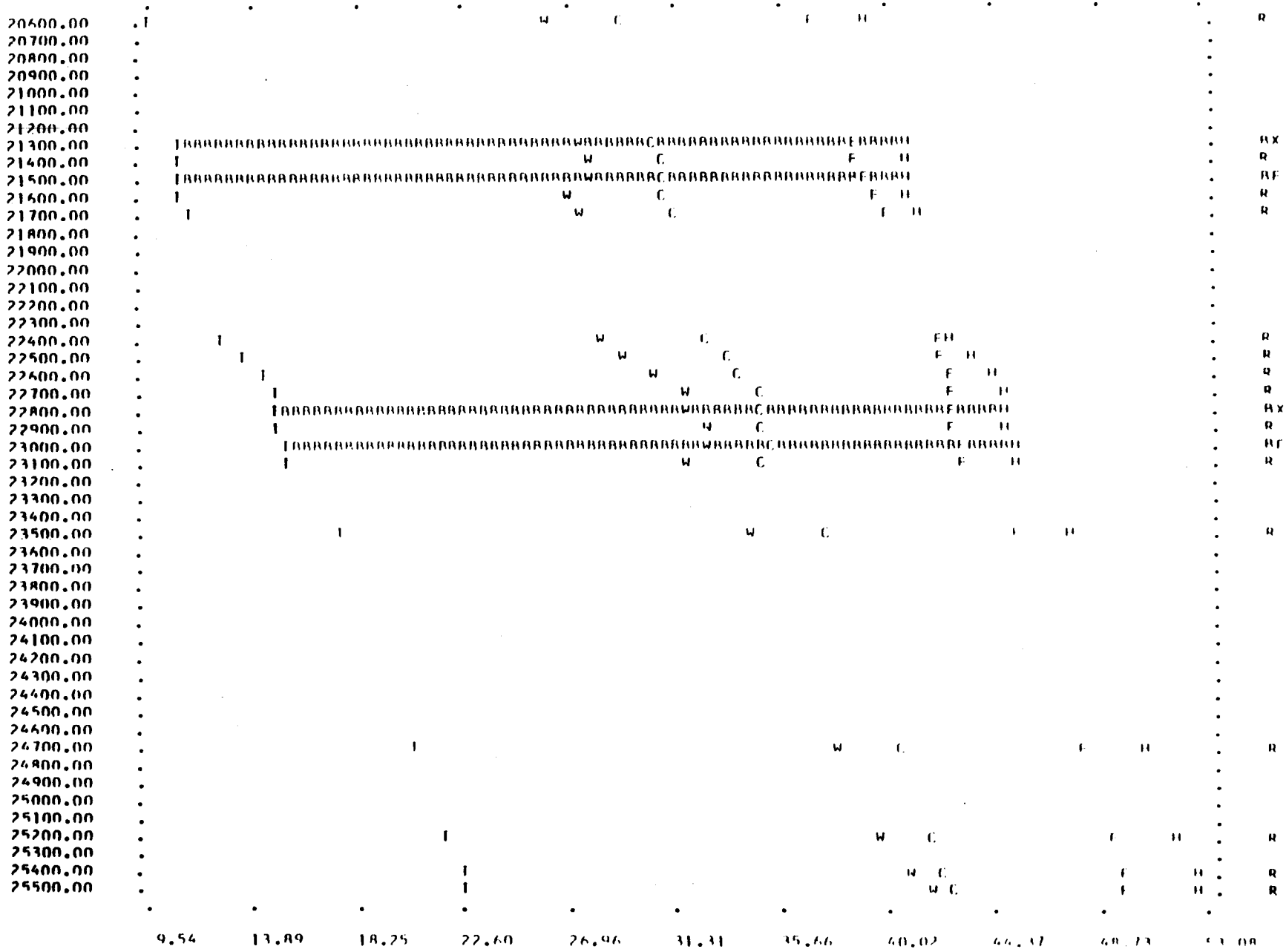
STATION / ITEM	INVERT ELEV / SO	DEPTH OF FLOW	W.S. ELEV	Q	VEL	VEL HEAD / SF AVE	ENERGY GRD.EL. / HF	SUPER ELEV	CRITICAL DEPTH / NORM DEPTH	HGT/ DIA	BASE/ ID NO.	ZL / ZR	NO PIER	AVGPR
20600.00	9.54	16.853	26.393	58500.0	26.59	10.976	37.369	0.0	19.830	30.00	80.00	3.00	0	0.0
700.00	0.00201					.002431	1.70		17.513			3.00		
21300.00	10.95	16.518	27.468	58500.0	27.34	11.604	39.072	0.0	19.840	30.00	80.00	3.00	0	0.0
BRIDGE EXIT												3.00		
21300.00	10.95	17.011	27.961	58500.0	26.76	11.116	39.077	0.0	20.055	30.00	80.00	3.00	1	2.50
80.00	0.00200					.002989	0.24		18.865			3.00		
21380.00	11.11	16.917	28.027	58500.0	26.96	11.289	39.316	0.0	20.055	30.00	80.00	3.00	1	2.50
BRIDGE ENTRANCE												3.00		
21380.00	11.11	16.136	27.246	58500.0	28.23	12.379	39.625	0.0	19.840	30.00	80.00	3.00	0	0.0
264.29	0.00201					.002827	0.75		17.520			3.00		
21644.29	11.64	15.949	27.591	58500.0	28.69	12.781	40.372	0.0	19.830	30.00	80.00	3.00	0	0.0
675.47	0.00201					.003096	2.09		17.520			3.00		
22319.76	13.00	15.404	28.404	58500.0	30.09	14.059	42.463	0.0	19.830	30.00	80.00	3.00	0	0.0
34.72	0.02000					.003138	0.11		9.497			3.00		
22354.48	13.69	15.829	29.523	58500.0	28.99	13.050	42.573	0.0	19.840	30.00	80.00	3.00	0	0.0
36.53	0.02000					.002790	0.10		9.497			3.00		
22391.01	14.43	16.386	30.811	58500.0	27.64	11.863	42.674	0.0	19.830	30.00	80.00	3.00	0	0.0
28.75	0.02000					.002443	0.07		9.497			3.00		
22419.76	15.00	16.960	31.960	58500.0	26.35	10.785	42.745	0.0	19.830	30.00	80.00	3.00	0	0.0
40.92	0.00269					.002275	0.09		16.253			3.00		
22460.68	15.11	16.982	32.092	58500.0	26.31	10.746	42.838	0.0	19.830	30.00	80.00	3.00	0	0.0
BRIDGE EXIT												3.00		

WATER SURFACE PROFILE LISTING

BALLONA CREEK , STA 204+00 TO 255+00
SAMPLE PROBLEM ENTERED BY SAMAN J. SHAHIN DATE : FEB 27 , 1979
TYPICAL TRAPEZOIDAL CHANNEL WITH PIERS .

STATION	INVERT ELEV	DEPTH OF FLOW	W.S. ELEV	Q	VEL	VEL HEAD	ENERGY GRD.EL.	SUPER ELEV	CRITICAL DEPTH		HGT/ DIA	BASE/ ID NO.	ZL	NO PIER	AVAPR
L/ELEM	SO					SF AVE	HF			NORM DEPTH			ZR		
22460.68	15.11	17.552	32.662	58500.0	25.61	10.182	42.844	0.0	20.055		30.00	80.00	3.00	2	1.25
100.00	0.00240					.003222	0.32			18.541			3.00		
22560.68	15.37	17.456	32.826	58500.0	25.81	10.340	43.166	0.0	20.055		30.00	80.00	3.00	2	1.25
BRIDGE ENTRANCE													3.00		
22560.68	15.37	16.566	31.936	58500.0	27.23	11.512	43.448	0.0	19.830		30.00	80.00	3.00	0	0.0
880.52	0.00262					.002449	2.16			16.357			3.00		
23441.20	17.68	16.738	34.418	58500.0	26.84	11.187	45.605	0.0	19.830		30.00	80.00	3.00	0	0.0
1164.95	0.00262					.002251	2.62			16.357			3.00		
24606.15	20.73	17.322	38.057	58500.0	25.59	10.170	48.227	0.0	19.830		30.00	80.00	3.00	0	0.0
497.35	0.00262					.001972	0.98			16.357			3.00		
25103.50	22.04	17.922	39.962	58500.0	24.40	9.246	49.208	0.0	19.830		30.00	80.00	3.00	0	0.0
248.62	0.00262					.001728	0.43			16.357			3.00		
25352.12	22.69	18.540	41.232	58500.0	23.27	8.405	49.637	0.0	19.830		30.00	80.00	3.00	0	0.0
116.02	0.00262					.001514	0.18			16.357			3.00		
25468.14	23.00	19.176	42.172	58500.0	22.18	7.641	49.813	0.0	19.830		30.00	80.00	3.00	0	0.0
31.86	0.00262					.001326	0.04			16.357			3.00		
25500.00	23.08	19.830	42.910	58500.0	21.15	6.945	49.855	0.0	19.830		30.00	80.00	3.00	0	0.0

BALTIMA CREEK , STA 20600 TO 25500
SAMPLE PROBLEM ENTERED BY SAMAN J. SHAHIN DATE : FEB 27 , 1979
TYPICAL TRAPEZOIDAL CHANNEL WITH PIERS .



N O T E S

1. GLOSSARY
   - I = INVERT ELEVATION
   - C = CRITICAL DEPTH
   - W = WATER SURFACE ELEVATION
   - H = HEIGHT OF CHANNEL
   - E = ENERGY GRADE LINE
   - X = CURVES CROSSING OVER
   - B = BRIDGE ENTRANCE OR EXIT
   - Y = WALL ENTRANCE OR EXIT
2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

9.1 ERROR MESSAGES AND EXPLANATIONS

9.1.1 ERROR MESSAGES IN EDITING THE ELEMENT CARDS

1) THE ABOVE SYSTEM OUTLET WAS FOUND TO BE IN ERROR - ELEMENT NOT EQUAL TO 001

The system outlet must be the first element to be processed. Check order of input to make sure the system outlet card follows the title cards.

2) THE ABOVE INPUT CARD DID NOT CONTAIN THE REQUIRED DATA XXX

Check the data on the input card with the input documentation for that element to make sure all the required data is present, see a programmer if data is correct.

3) THE ABOVE INPUT CARD CONTAINED AN INVALID ELEMENT NUMBER

The above card contained an invalid code in the element type field. Check input field with input documentation description and correct data.

4) THE ABOVE INPUT CARD CONTAINED AN INVALID STATION

The station on the above card was not in sequence with the previous stations. The value was less than the station of the previous element. Correct input data.

5) THE ABOVE INPUT CARD CONTAINED AN INVALID INVERT

6) DURING EDIT PHASE XXX ERRORS WERE ENCOUNTERED - PROCESSING WILL NOT START

The number of errors in the edit phase is printed. The calculations will not begin until all edit errors have been corrected.

7) NO EDIT ERRORS ENCOUNTERED - COMPUTATION IS NOW BEGINNING

This message indicates that all of the input data was correct and processing calculations will begin.

8) A BLANK INVERT WAS GIVEN ON AN ELEMENT CARD

The above card was required to have an invert but none was given, check and correct input data.

9) INVALID SECTION NUMBER ON ELEMENT CARD

The section number given for this element is not between 1 and 200. Correct input with correct channel number reference.

10) SECTION NUMBER HAD NO DATA FOR CHANNEL DEFINITION

The section number on the element card refers to a section that was not defined or that was labeled as being in error in editing the channel definition cards. Check the channel number and the results from the channel definition editing.

11) SECTION NUMBER HAD NO DATA FOR CROSS SECTION

Same as Message No. 10 only for cross section data instead of channel definition data.

12) THE CHANNEL DEFINITION REFERENCED DID NOT CONTAIN THE REQUIRED DATA TO BE USED IN THIS ELEMENT

There is a conflict between the data in the channel definition used to describe this element and the type of element being described. Check the restrictions for this element and make sure the channel definition selected has applicable data.

13) THE PREVIOUS SECTION OR CHANNEL DEFINITION DID NOT COINCIDE WITH THE DATA UTILIZED IN THIS ELEMENT

There is a conflict between the data in the channel defintions of the previous element and the current element being used in the element being described. Check the restrictions for the element type and the channel definition data used.

9.1.2 WARNING MESSAGES IN EDITING THE ELEMENT CARDS

1) THE ABOVE ELEMENT CONTAINED AN INVERT ELEV WHICH WAS NOT GREATER THAN THE PREVIOUS INVERT ELEV.

   Check the inverts on the preceeding and current elements to make sure they are what you want. Program assumes data is good and continues.

2) WARNING - ADJACENT SECTIONS ARE NOT IDENTICAL - SEE SECTION NUMBERS AND CHANNEL DEFINITIONS

   The two adjacent sections are supposed to be identical, but if only the channel height varies it is alright. Check channel definitions to see if data is correct and only height varies, program assumes data is correct and continues.

3) WARNING - PREVIOUS SECTION NUMBER WAS INVALID OR 0 - SEE PREVIOUS DESCRIPTION

   Previous element should have been flagged as being bad so the data passed to this element is zeroes. Processing for the other elements continues.

9.1.3 ERROR MESSAGES IN SEQUENCE CHECKING THRU CHANNEL DEFINITION DATA

1) NO SYSTEM HEADWORKS CARD - CANNOT TELL WHERE THE START OF CHANNEL DEFINITION DATA IS - NO PROCESSING

There must be a system headworks card at the end of the element cards just preceeding the channel definition cards. Check input data.

2) CHANNEL DEFINITION DATA (CD) DID NOT FOLLOW THE SYSTEMS HEADWORKS CARD - CONTINUING TO LOOK FOR CD OR PTS

There must be at least one CD card following the system headworks card, and all CD cards follow the system headworks and come before the cross section points (PTS) cards. Check input data.

3) NO CHANNEL DEFINITION (CD) OR CROSS SECTION POINT CARDS (PTS) WERE RECOGNIZED - CHECK DATA

There must be at least one channel definition card following the system headworks card. Check input data card code columns.

4) NO CHANNEL DEFINITION CARDS BEFORE CROSS SECTION POINT CARDS - CHECK DATA

Check order of input cards. Element cards ending with system headworks must be followed by at least one channel definition card. Cross section point cards follow the channel definition cards.

5) INVALID CHANNEL TYPE ON CHANNEL DEFINITION CARD ITYPE = X SECT = XXX

ITYPE is the channel type requested and SECT is the section number the channel type is specified to define. Channel type must be a number between 1 and 6. Check and correct this CD card.

6) NO CROSS SECTION POINTS ENCOUNTERED - ASSUME NO IRREGULAR CHANNELS

No irregular channels or cross section points are indicated for this problem. This is a warning message. Processing will continue.

7) INVALID CARD CODE ENCOUNTERED WHILE PROCESSING CD AND PTS CARDS CODE = XXX

After the first CD card a card was found which did not have a code of CD or PTS. CODE indicates the invalid card code which should be corrected or placed in the correct order.

8) NO SYSTEM HEADWORKS CARD BEFORE CHANNEL DEFINITION OR CROSS SECTION POINTS

The system headworks card was omitted or is out of sequence. It should be the last element card and should immediately preceed the channel definition cards.

9.1.4 ERROR MESSAGES IN SEQUENCE CHECKING CROSS SECTION POINTS CARDS

1) INVALID OR MISSING NUMBER OF POINTS VALUE - MUST BE BETWEEN 3 AND 99. CODE = XXX ISECT = XXX NO PTS = XX

The number of points value is in error or the card is out of sequence, make sure this is supposed to be the first card of a cross section for the section points. CODE is the card code, ISECT is the section number and NO PTS is the number of points indicated. Correct the invalid card and resubmit.

2) INVALID CARD CODE FOR CROSS SECTION POINTS

While processing PTS cards a code not equal to PTS was found. Either wrong or out of sequence. Correct the data error.

3) STARTED ANOTHER CROSS SECTION GROUP BEFORE PREVIOUS GROUP WAS COMPLETED.

A new cross section group was indicated (no of points was given) before all the points indicated by the previous the number of points were read. Check card sequencing and make sure the number of points is only on the first card of a section and is correct with the number of points to be read.

4) SECTION NUMBER IS INVALID OR MISSING - MUST BE BETWEEN 1 AND 200 CODE = XXX ISECT = XXX NO PTS = XXX

The section number given is in error. CODE is the card code, ISECT is the invalid section no. and NO PTS is the number of points. Correct the invalid cross section points card and resubmit.

5) END OF FILE BEFORE ALL POINTS WERE READ ON LAST CROSS SECTION

The last input card was read before all the points indicated to exist in the current cross section were read. Supply the remaining cross section points cards to complete the section or correct the number of points indicated to define the section.

6) END OF FILE ON CROSS SECTION POINTS

The last cross section points card was read and processing in this program is completed.

7) NO CHANNEL DEFINITION RECORD FOR THIS SECTION CODE - GOING ON TO NEXT CROSS SECTION SECT = XXX

There was no channel definition or an invalid channel definition was given at the corresponding section number so no processing is done on these cross section points.

8) MISSING NUMBER OF POINTS FOR CODE XXX FOR SECTION XXX

The first card to describe the cross section points of a section did not have the number of points value to indicate how many points are to be read to describe the channel. Make sure this is supposed to be the first card of the section points and supply the number of points value for the section; if it is not supposed to be the first card of the section points put the cards in their proper sequence.

9.1.5 ERROR MESSAGES IN CHANNEL DEFINITION PROCESSING

1) SECTION NUMBER INVALID OR MISSING, DATA CANNOT BE WRITTEN TO THE OUTPUT FILE

   There was an invalid section number on a CD or PTS card. Section number must be between 1 and 200. Correct invalid data.

2) INVALID VALUE FOR THE NUMBER OF PIERS - MUST BE BETWEEN 0 AND 10 IF GIVEN

   The number of piers on the CD card is invalid. Correct the invalid data.

3) AVERAGE WIDTH OF PIERS IS INVALID OR NOT GIVEN WHEN THERE IS A VALUE FOR NUMBER OF PIERS IN THE CHANNEL

   When number of piers is given there must be a value for average width of piers. Correct whichever field is wrong.

4) CHANNEL HEIGHT IS INVALID OR IS NOT GIVEN

   Correct the height data in the channel definition.

5) CHANNEL DIAMETER IS INVALID OR IS NOT GIVEN

   Correct the width data in the channel definition.

6) CHANNEL WIDTH IS INVALID OR IS NOT GIVEN

   Correct the width data in the channel definition.

7) THERE IS A DIFFERENCE BETWEEN THE NO. OF PIERS AND THE NUMBER OF VALUES FOR PIER DEPTHS

   If fewer depths are given for piers in an irregular section than the number of piers indicated, then the remaining pier base values must be added even if they are 0.0. If more depths are given only the amount up to the number of piers declared will be considered.

9.1.6 ERROR MESSAGES IN CROSS SECTION POINT PROCESSING

1) ENCOUNTERED A POINT WHERE X = 0 AND Y = 0 BEFORE ALL THE INDICATED POINTS WERE PROCESSED - ASSUMING ERROR

Only the first coordinate of the cross section points can be 0, 0 - otherwise the program cannot distinguish between blanks and zeroes. If point desired is 0, 0 use .01, .01 for approximate data. Correct invalid data.

2) THE CROSS SECTION POINTS ARE OUT OF SEQUENCE FOR AN IRREGULAR OPEN SECTION - MUST BE COUNTER - CLOCKWISE FROM MINIMUM X

Check the sequence of points on the cross section point cards for the data which is out of order.

3) THE CROSS SECTION POINTS ARE OUT OF SEQUENCE FOR AN IRREGULAR SECTION - MUST BE COUNTER - CLOCKWISE FROM MINIMUM X

Check the sequence of points on the cross section point cards for the data which is out of order. When maximum X is reached the following X values must continually decrease.

4) MAXIMUM Y IS NOT AT EITHER SIDE OF AN OPEN IRREGULAR CHANNEL ASSUMED BAD DATA AND PROCESSING IS STOPPED

For some reason maximum Y was not at the end of an open irregular channel. Check input data and correct if wrong, otherwise see programmer.

9.1.7 COMPUTATION ERROR AND WARNING MESSAGES

1) INVALID RECORD CODE IN XXX, IREC = X, ELEMENT = XXX;

The subroutine, record code, and element number are printed. An invalid record code was encountered, the record code value should be between 1 and 9. This code is set internally in the edit phase so it is a program problem - see programming staff. Processing is stopped.

2) WATER SURFACE ELEVATION GIVEN IS LESS THAN OR EQUALS INVERT ELEVATION IN XXX, W.S. ELEV = INV + DC;

The subroutine name is printed. This is a warning message that there was no water surface elevation inputted for either the headworks or outlet or that the water surface inputted is less than the invert elevation causing DC to be the controlling depth. Processing continues.

3) WENTDS, NO AREA OF OBSTRUCTION IN ELEMENT XXX, A1 = XXX, A2 = XXX;

The element number, area in the U/S end, and area in the D/S end (based on depth from the U/S end) are printed. The area in the U/S end must be greater than the area in the D/S end. Make sure this is supposed to be a wall entrance and that the channel sections are described properly. Processing is stopped.

4) W.S. ELEV. IS 10 FEET OR MORE ABOVE OPEN CHANNEL WALLS IN XXX, STATION = XXX, D = XXX, DH = XXX;

The subroutine, station, depth, and maximum open flow depth are written. Open flow depth reached the maximum limit in the program. Raise the heights of the channel walls at this point and resubmit. Processing is stopped.

5) OVER 50 RECORDS WRITTEN IN XXX ELEMENT = XXX STATION = XXX;

The subroutine, element, and station are printed. The maximum number of 50 intermediate points in a reach element have been processed. Divide this reach element into two or more reaches at the station printed and resubmit. Processing is stopped.

6) CANNOT SOLVE QUADRATIC FORMULA FOR START OF OPEN FLOW IN RCHUS, STATION = XXX;

The station at the D/S end of the reach is printed. The solution to solving the quadratic formula was negative for the length of reach in pressure flow. There is no solution for this problem (it should not occur), if it does this element must be hand calculated and the other elements can be run with the hand calculated control depths. Processing is stopped.

7) OTLTJP HAS ERRONEOUS PROCESSING CODES, U/S CODE = X, D/S CODE = X, D = XXX, DC = XXX;

The U/S and D/S processing codes, depth, and critical depth are written. There must be at least U/S processing in the outlet. This is an internal program error because the processing codes for the outlet are improperly set. See programmer. Processing is stopped.

8) A STATION ON THE U/S OR D/S FILE IS PAST THE END OF THE REACH IN RCHJP, STATION FROM FILE = XXX, REACH STATION = XXX;

The station from the U/S or D/S file and the station at the U/S or D/S end of the reach are printed. The station on the file is not between the U/S and D/S ends of the reach. This is an internal problem, a station computed in U/S or D/S processing is in error or the value for the number of U/S or D/S records written is in error. See programmer. Processing is stopped.

9) THE KNOWN DEPTH EQUALED THE NORMAL DEPTH IN BERNLI, DEPTH = XXX;

The known depth is printed. This is a warning message that normal depth has already been reached. The depth at the end of the reach is set equal to normal depth. Processing continues.

10) THE UPPER AND LOWER LIMIT VALUES CALCULATED IN BERNLI WERE THE SAME, LOWER LIMIT = XXX, UPPER LIMIT = XXX;

The values from Bernoulli's equation based on the lower and upper limit depths are printed. This is a warning message indicating that depth cannot be found by Bernoulli's equation and that the upper and lower limit depths are the same. The depth at the end of the reach is set equal to the current known depth. Processing continues.

11) THE VALUE TO SOLVE FOR DEPTH IN BERNLI IS NOT BETWEEN THE UPPER AND LOWER VALUE LIMITS, DESIRED VALUE = XXX, UPPER LIMIT VALUE = XXX, LOWER LIMIT VALUE = XXX, UPPER LIMIT DEPTH = XXX, LOWER LIMIT DEPTH = XXX;

The value needed to solve Bernoulli's equation, the upper and lower limit values from Bernoulli's equation, and the upper and lower limit depths are printed. This is a warning message indicating that depth to solve Bernoulli's equation cannot be found between the limits where it is expected. Depth at the end of the reach is set to the current known depth or to normal depth depending on whether the desired value to solve Bernoulli's equation is greater or less than the prescribed limits. Processing continues.

12) THE XX FILE DOES NOT HAVE DEPTH AT THE HYDRAULIC JUMP IN JUMPR;

The U/S or D/S file is printed. The station of the hydraulic jump cannot be computed although it is indicated to exist because the U/S and D/S force curves crossed. The actual location of the jump is not included on either the U/S or D/S file. This is an internal problem and should not happen. Check U/S and D/S reach processing to see if they are valid (print switch = 2). See programmer. Processing is stopped.

13) NO INTERSECTION OF FORCE CURVES COULD BE FOUND FOR THE HYDRAULIC JUMP IN JUMPR;

A hydraulic jump was indicated but there was insufficient data on the U/S and D/S files to locate the point of intersection. This is an internal problem and should not occur. Check U/S and D/S reach processing to see if they are valid (print switch = 2). See programmer. Processing is stopped.

14) THE FORCE AT THE HYDRAULIC JUMP IS NOT BETWEEN THE FORCES FROM THE UPPER AND LOWER LIMIT DEPTHS, UPPER LIMIT DEPTH = XXX, LOWER LIMIT DEPTH = XXX, UPPER LIMIT FORCE = XXX, LOWER LIMIT FORCE = XXX, FORCE AT JUMP = XXX IN PPMDEP;

The upper and lower limit depths (depth from either side of indicated hydraulic jump), the upper and lower limit forces, and the force at the hydraulic jump are written. The force at the jump should be equal or between the forces on either side of the jump but this was not the case. Either the force given for the jump or the points given from the U/S or D/S file adjacent to the jump are wrong. Check the U/S and D/S files for valid data (print switch = 2). This is an internal problem. See programmer. Processing is stopped.

15) THE TEST DEPTH EXCEEDED THE UPPER LIMIT DEPTH BEFORE THE FORCE AT THE JUMP WAS REACHED, TEST DEPTH = XXX, UPPER LIMIT DEPTH = XXX, TEST FORCE = XXX, JUMP FORCE = XXX IN PPMDEP;

The iterated depth, upper limit depth, iterated force, and force at the hydraulic jump are printed. The depth causing the force at the hydraulic jump should be equal or between the depths on either side of the jump but this was not the case. Either the force given for the jump or the points given from the U/S or D/S file adjacent to the jump are wrong. Check the U/S and D/S files for valid data (print switch = 2). This is an internal problem. See programmer. Processing is stopped.

16) ELEMENT NO. XXX HAS DNORM OR DCRIT LESS THAN OR EQUAL TO ZERO IN ELMCHG;

The element number is printed. Either normal depth or critical depth could not be computed for this reach. This should not happen, check the channel description for the reach element. Hand calculate normal and critical depths and if they exist in the channel description see a programmer because either function DNORM or function DCRIT is in error. This exists as a warning message and processing will continue but will probably terminate before the end of the run.

17) XXX ERRORS WERE ENCOUNTERED IN SETTING THE PRELIMINARY VALUES IN ELMCHG;

The number of errors in analyzing adjacent elements and flow rates and computing critical and normal depths in written. These errors must be corrected and the program must be rerun before actual processing will start. If this message occurs on the same run for other than the first flow rate there is an internal problem, then see the programmer. Processing is stopped.

18) NO XX RECORDS EXISTED WHERE INDICATED - ELEMENT NO. XXX IN WRITEN;

The U/S or D/S file and the element number are printed. The U/S or D/S processing code indicated the computation for the element was valid but there were no records on that file for the element. This is an internal problem with the processing codes. See programmer. Processing continues with the next element.

19) THERE WAS NO JUMP INDICATED WHEN BOTH U/S AND D/S RECORDS EXISTED FOR ELEMENT XXX IN WRITEN;

The element number is printed. There was a problem in the jump processing for this element. Either one of the profiles should be deleted or a hydraulic jump should be indicated. This is an internal problem, see programmer. Processing continues with the next element.

20) A JUMP WAS INDICATED BUT THERE WERE NOT RECORDS ON BOTH THE U/S AND D/S PROFILES FOR ELEMENT XXX IN WRITEN;

The element number is printed. There was a problem in the jump processing for this element. If the entire U/S or D/S profile is deleted then there cannot be a jump and if there is a jump there must be U/S and D/S profile data. This is an internal problem, see programmer. Processing continues with the next element.

21) THERE WERE NO RECORDS FOR ELEMENT XXX IN WRITEN;

The element number is printed. This is a warning message to indicate there was no U/S or D/S processing for this element. Check the U/S and D/S profiles (print switch = 2) to verify this. If there is data there is an internal problem, if there is no data check the construction of the element. Processing continues with the next element.

22) NO PLOT GENERATED, BAD DATA OR NOT ENOUGH POINTS, 3 OR LESS;

If there are only 3 elements being run no plot will be generated, otherwise there was a problem in processing one of the elements and there is an internal problem. This is a warning message and processing continues.

23) ELEMENT NUMBER XXX HAS ADJACENT ELEMENTS WHICH ARE IN ERROR;

The element number is printed. There is an error in the sequence of elements submitted (such as bridge exits back to back) which are not allowed. Check the sequence of the elements, correct the error, and resubmit the data. This is a user error, sequence checking will continue but actual processing will be stopped.

24) XXX DEPTH COULD NOT BE FOUND IN ELEMENT XXX;

Either normal or critical depth and element number are printed. There is either an error in function DCRIT or DNORM or there is a bad channel description. Hand calculate the value and if it is valid for the channel see a programmer. The elements will continue to be checked but no actual processing will take place until the error is resolved.

25) IRREGULAR XXX VALUES ARE ZERO OR NEGATIVE, SET XXX EQUAL TO ZERO, XXX = XXX, PIER XXX = XXX, IN XXX;

Either force, area, or wetted perimeter values are printed from functions FORCEI, AREACI, or WETPI for irregular sections. The appropriate data could not be computed in this irregular section. The problem is internal. The cross section points used for the computation are probably distorted. See a programmer. The desired value is set to zero and processing continues but will probably be in error and processing will probably terminate before the end of the run.

26) PIER WIDTH IS WIDER THAN CHANNEL WIDTH IN XXX, DEPTH = XXX, PIER WIDTH = XXX;

Either force, area, or wetted perimeter, depth, and average pier width is printed. The width of the number of piers at the given depth is wider than the channel width at that depth. This is a user error. Correct the input data and resubmit. The desired value is set to zero and processing continues but results will probably be erroneous and the program will probably terminate before the end of the run.

27) DEPTH EXCEEDS XXX WITH FORCE TOO LOW IN FORCEM, TEST DEPTH = XXX, TEST FORCE = XXX, XXX = XXX, DESIRED FORCE = XXX;

The iterated depth and force, the maximum or minimum depth, and the desired force value are printed. The desired force in the bridge exit could not be reached within the prescribed depth limits. The desired depth is set to zero and no processing is done in that end of the bridge exit. Processing continues with the next element.

28) DESIRED FORCE IS OUT THE RANGE OF DEPTHS IN FORCEF, TEST DEPTH = XXX, TEST FORCE = XXX, XXX = XXX, DESIRED FORCE = XXX;

The iterated depth and force, the maximum or minimum depth, and the desired force are printed. In D/S processing the desired force in the D/S end of the bridge entrance could not be reached within the prescribed limits of depth so the desired depth is set to zero and no computation is done in the D/S end. In U/S processing the bridge entrance was under pressure at the U/S end so pressure flow calculations will be done. Processing is continued for pressure flow going U/S and in the next element going D/S.

29) DESIRED FORCE IS OUT OF THE RANGE OF DEPTHS IN FWALL, TEST DEPTH = XXX, TEST FORCE = XXX, MINIMUM DEPTH = XXX, DESIRED FORCE = XXX;

See Message 28 for D/S bridge entrance only use wall entrance instead.

30) DEPTH IS OUTSIDE THE RANGE OF THE POINTS DESCRIBING THE CHANNEL IN XXX, DEPTH = XXX, YMIN = XXX, YMAX = XXX;

Either force, area, or wetted perimeter values for depth and minimum and maximum Y values are printed. If the depth is not above maximum open flow depth there is an internal error, see programmer. If the depth exceeds maximum open flow depth raise the channel walls. The desired value is set to zero and processing continues but will probably be in error and processing will probably terminate before the end of the run.

31) UNABLE TO CALCULATE FRICTION SLOPE WITH MANNINGS EQUATION IN SF, AREA = XXX, WETTED PREIMETER = XXX;

The area and wetted perimeter are printed. Either the area or wetted perimeter should be less than or equal to zero. This is an internal problem, see programmer. Processing is stopped.

32) CRITICAL DEPTH MAY BE INACCURATE IN ELEMENT XXX, INCREMENT = XX;

The element number and increment value are printed. If the increment is large then critical depth is probably above the top of the channel but is set equal to the channel height. If the increment is small critical depth is probably pretty accurate but for some reason it cannot be computed precisely. This is a warning and processing continues.

33) Q VALUES IN THE JUNCTION ARE INCORRECT FOR DEPSMP, Q1 = XXX, Q2 = XXX, Q3 = XXX, Q4 = XXX;

The Q values for both U/S and D/S ends and for the laterals are printed. Q2 should equal the sum of the other Q's. If it does not there is an internal error in subroutine ELMCHG, see a programmer. If these Q values are in error, resubmit the input data with the correct Q values. Processing is stopped.

34) A LATERAL ANGLE OF CONFLUENCE IS GREATER THAN 90 DEGREES IN DEPSMP, FIRST ANGLE OF CONFLUENCE = XXX, SECOND ANGLE OF CONFLUENCE = XXX;

The angles of the laterals are printed. This is a user error, check the values inputted for the angles of the laterals, they should be in degrees. Processing is stopped.

35) INVALID PROCESSING CODE WAS ENCOUNTERED IN XXX, PROCESSING CODE = X, AND IT SHOULD BE 1 OR 2;

Function DEPSMP, SUMM, or SUMP is printed with the processing code. The processing code should be 1 for D/S processing and 2 for U/S processing representing the known end of the element. This code is internally set just before the function is called so it is a programming problem. Processing is stopped.

36) MOMENTUM AND PRESSURE CURVES DID NOT CROSS IN DEPSMP, SETTING DEPTH EQUAL TO UPPER LIMIT DEPTH PLUS ONE FOOT, DEPTH = XXX;

The depth is printed. The intersection of the pressure and momentum curves was above the maximum open flow depth. For a closed channel pressure flow calculations will be executed, otherwise, processing will stop because depth is too high in an open channel. This is a warning message.

9.1.8 DEBUG MESSAGE

NO COMPUTATION IN XXXX - A BREAK IN WATER SURFACE PROFILE, STATION = XXX, XXXX

The station, subroutine, and paragraph number are printed. This is a notification message which identifies whether there was any computation for the U/S or D/S end of an element.

Other messages printed in debug trace have the subroutine or function and paragraph number printed.

10.1 SYSTEM RECOVERY PROCEDURE

If any job fails to process to a normal end for any reason, please contact the Data Processing Section and bring all pertinent information with you to aid in solving the problem.

11.1 PROGRAM LIMITATIONS

1) A maximum of 200 elements are allowed per run.

2) A maximum of 50 intermediate points can be computed in a reach element.

3) Critical depth cannot exceed 100 feet.

4) Program will not compute the water surface profile when the friction slope is at one or greater.

5) Open channel processing is limited to a depth ten feet above the height of the described element.

6) Undulating bottoms cannot be calculated properly in an irregular shaped section unless the depth of flow is above the undulations.

7) The program will not accept vertical drops in invert elevations.

8) Calculations in the Water Surface profile print out may be slightly inaccurate (to .001) due to rounding variables to be contained on the U/S and D/S data files.

9) The invert cross fall "e" in inches is optional and may be used only with channel type two, a rectangular open channel or R.C. Box Sections.

12. APPENDIX

This Appendix contains the computational symbols, methods, procedures, and equations used in the program.

Table of Contents

		Page
12.1	Symbol Definitions	91
12.2	Irregular Section Definitions	93
12.3	Conduit Section Geometry Equations	94
	12.3.1 Flow Area	95
	12.3.2 Wetted Perimeter	96
	12.3.3 Hydrostatic Pressure	97
	12.3.4 Depth of Flow	98
12.4	Computational Procedures	99
	12.4.1 Basic Equations of Steady Flow	99
	12.4.2 Reach Analysis	101
	12.4.3 Transition Analysis	102
	12.4.4 Junction Analysis	102
	12.4.5 Pier Analysis	103
	12.4.6 Wall Entrance Analysis	104
	12.4.7 Wall Exit	104
12.5	Pictorial of Input Deck	105
12.6	References	106

12.1 SYMBOL DEFINITIONS

Symbol	Definition
A	Cross sectional area of flow
Ap	Cross sectional area of pier
b	Base width of channel
bnet	Net base width of channel
bp	Average base width of piers
D	Depth of flow
Dc	Critical depth
L	Length between two stations
H	Drop in invert between two points
DH	Maximum open flow depth in a section: Ten feet above channel height in open section, Height of channel in closed section
DN	Normal depth
E	Specific Energy
EGL	Energy Grade Line elevation
e	Invert cross fall in inches may be used with channel type two only (optional)
F	Force
g	Gravitational constant (32.2 ft/sec/sec/)
HAPT	Head loss due to angle point
HB	Head loss due to bend or curve
HF	Head loss due to friction
HJ	Head loss in a junction
HMH	Head loss in a manhole
Ht	Head loss in a transition
HV	Velocity head
INV	Invert elevation in a channel section
M	Momentum
n	Manning's n, coefficient of roughness
No P	Number of piers in a section (max. of 10)
P	Hydrostatic Pressure
Q	Flow rate
R	Radius of pipe
RH	Hydraulic radius
r	Radius of curve on horizontal alignment
SF	Friction slope (energy loss per foot)
Sc	Critical slope (slope at critical depth)
So	Invert slope
SE	Super elevation
STA	Station
V	Velocity
WP	Wetted perimeter
WS/HGL	Water Surface or Hydraulic Grade Line Elevation
ZL	Left side slope
ZR	Right side slope

These variables may have one of two suffixes

1: Identifies the variable at the upstream end of an element.
2: Identifies the variable at the downstream end of an element.

Example: V1: is velocity at the upstream end
V2: is velocity at the downstream end.

Throughout this program: U/S is the upstream end
D/S is the downstream end

12.2 IRREGULAR SECTION DEFINITIONS

Points i(X, Y) from i = 1 to i = n, in counter clockwise direction ($3 \leq n \leq 99$) define an irregular cross section.

X(i) : X coordinate of point(i)

Y(i) : Y coordinate of point(i)

Y(p) : Y coordinate of base of pier, from P = 1 to P = n given from left to right ($0 \leq n \leq 10$)

Δ X(i) = X(i) - X(i-1)

Δ Y(i) = Y(i) - Y(i-1)

Ymin = The least Y(i)

Y(i) = Y(i)-Ymin, subtract Ymin from every Y(i) on file

Y(P) = Y(P)-Ymin, subtract Ymin from every Y(P) on file

Ymax : Maximum open flow depth in a cross section
All points whose Y(i) > Ymax will be dropped from file

Y(D) : Depth of flow in a section part full or under pressure

DH = Ymax + 10' in an open top section

DH = Ymax in a covered section

Point iBEG(X, Y): Contact Point of W.S. and the left bank.

If [$Y(D) \geq$ Ymax] then Y(iBEG) = Ymax

and X(iBEG) is least X for Ymax

If [$Y(D) <$ Ymax] then Y(iBEG) = Y(D)

$$X(iBEG) = [\Delta X(i+1) / \Delta Y(i+1)] [Y(D)-Y(i)] + X(i)$$

where Y(i) and Y(i+1) are the domain of Y(D) from Y(1) to Ymin.

Point iEND (X, Y): Contact Point of W.S. and the right bank.

If [Y(D) $\geq$ Ymax] then Y(iEND) = Ymax

and X(iEND) is the maximum X with Ymax.

If [Y(D) $<$ Ymax] then Y(iEND) = Y(D)

$$X(iEND) = [\Delta X(i+1) / \Delta Y(i+1)] [Y(D)-Y(i)] + X(i)$$

where Y(i-1) and Y(i) are the domain of Y(D) from Ymin to Y(n).

Let Point 1 (X, Y) = Point iBEG (X, Y)

where X(1) = X(iBEG)
Y(1) = Y(iBEG)

Let Point n (X, Y) = Point iEND (X, Y)

where X(n) = X(iEND)
Y(n) = Y(iEND)

Also Point n+1 (X, Y) = Point 1 (X, Y)

where X(n+1) = X(1)
Y(n+1) = Y(1)

12.3.1 AREA (A)

The Area (A) of flow is a function of the depth of flow and the geometry of the channel or conduit section.

1. CHAN. TYPE 1:

$$A = D\ [bnet + 0.5\ D\ (ZL+ZR)]$$

2. CHAN. TYPE 2:

$$A = [D \cdot bnet] - [e \cdot bnet/24]$$

3. CHAN. TYPE 3:

If D is less than Chan. height (H) see Process 1. Otherwise:

$$A = H\ [bnet + 0.5\ H\ (ZL+ZR)] - [0.25\ (1 + NOP)]$$

4. CHAN. TYPE 4:

If D is equal or greater than Diameter

$$A = \pi\ (R)^2$$

otherwise

$$A = (D-R)\ \sqrt{2RD - (D)^2} + [\pi\ (R)^2\ /180]\ [\text{Arcsin}\ ((D-R)/R)] + \pi\ (R)^2/2$$

5. CHAN. TYPE 5:

$$A = \sum_{i=2}^{i=n+1} \Delta X(i)\ [Y(D) - Y(i)] + 0.5[\Delta X(i) - \Delta Y(i)] - \sum_{P=1}^{P=n} (bp)\ [Y(D) - Y(P)]$$

6. CHAN. TYPE 6:

See Process 5.

12.3.2 WETTED PERIMETER (WP)

WP is a function of the depth of flow and the geometry of the conduit or channel section.

1. CHAN. TYPE 1:

$$WP = D\left[\sqrt{1 + (ZL)^2} + \sqrt{1 + (ZR)^2} + (2 \cdot NOP)\right] + bnet$$

2. CHAN. TYPE 2:

$$WP = 2\,D\,(1 + NOP) + bnet$$

3. CHAN. TYPE 3:

If D is less than chan. height (H) see Process 1. Otherwise

$$WP = D\left[\sqrt{1 + (ZL)^2} + \sqrt{1 + (ZR)^2} + (2 \cdot NOP)\right] + [2bnet] + [H\,(ZL + ZR)]$$

4. CHAN. TYPE 4:

If D is equal or greater than pipe diameter (2R) then

$$WP = 2\pi R$$

Otherwise

$$WP = [\pi R/90]\,[\text{ARCCOS}\,((R-D)/R)]$$

5. CHAN. TYPE 5:

$$WP = \sum_{i=2}^{i=n} \sqrt{[\Delta X(i)]^2 + [\Delta Y(i)]^2} + \sum_{P=1}^{P=n} 2\,[Y(D) - Y(P)] - bp$$

6. CHAN. TYPE 6:

If D is less than Ymax see Process 5. Otherwise

$$WP = \sum_{i=2}^{i=n+1} \sqrt{[\Delta X(i)]^2 + [\Delta Y(i)]^2} + \sum_{P=1}^{P=n} 2\,[Y(D) - Y(P)] - bp$$

12.3.3 HYDROSTATIC PRESSURE (P)

Pressure is a function of the depth of flow and the geometry of the conduit or channel section.

1. CHAN. TYPE 1:

$$P = 0.5\,(D)^2\,[bnet + D\,(ZL + ZR)/3]\,, \quad \text{where } bnet = b - (NOP \cdot bp)$$

2. CHAN. TYPE 2:

$$P = 0.5\,[bnet\,(D)^2\,]$$

3. CHAN. TYPE 3:

If D is not greater than chan. height (H) see Process 1. Otherwise:

$$P = 0.5\,(D)^2\,[bnet + D\,(ZL+ZR)/3] - 0.5\,(D-H)^2\,[(1/3)(ZL+ZR)(2H + D) + bnet]$$

4. CHAN. TYPE 4:

If D is equal or greater than the pipe diameter then

$$P = (D-R)\,\pi\,(R)^2$$

Otherwise: $C = (D/R)$ and:

$$P = (R^3/3)\left\{[(C)^2 - (2C) + 3]\,\sqrt{2C - (C)^2} + [\frac{\pi}{60}(C-1)]\,[90 + \text{ARCSIN}\,(C-1)]\right\}$$

5. CHAN. TYPE 5:

$$P = 0.5\sum_{i=2}^{i=n+1}\Delta X(i)\,[D - Y(i)]^2 + \Delta X(i)\,\Delta Y(i)\,[D - Y(i) + \Delta Y(i)/3] - \sum_{P=1}^{P=n} 0.5\,bp\,[D - Y(P)]^2$$

6. CHAN. TYPE 6:

If D is not greater than Ymax see Process 5. Otherwise:

$$P = \sum_{i=2}^{i=n+1} 0.5\,\Delta X(i)\,[D - Y(i)]^2 + 0.5\,\Delta X(i)\,\Delta Y(i)\,[D - Y(i) + \Delta Y(i)/3] - \sum_{P=1}^{P=n} bp\,[Ymax - Y(P)]\,[D - 0.5\,Ymax - 0.5\,Y(P)]$$

12.3.4 DEPTH OF FLOW (D)

Depth computation when area of flow is known.

1. CHAN. TYPE 1:

$$D = \frac{1}{Z + Z}\left\{-bnet + \sqrt{(bnet)^2 + 2(ZL + ZR)\ [A]}\right\}$$

2. CHAN. TYPE 2:

D = (1/bnet) [A] + (e.bnet/24)

3. CHAN. TYPE 3:

Compute AH (The Full Area of a closed section).

If A is less than AH see Process 1. Otherwise:

D = HGL - Inv.

4. CHAN. TYPE 4:

Compute AH.

If A is not less than AH

D = HGL - Inv.

Otherwise

Find D by trial and error from Area of part full pipe.

5. CHAN. TYPE 5:

Find D by trial and error from Area of part full section.

6. CHAN. TYPE 6:

Compute AH

If A is not less than AH then

D = HGL - Inv

Otherwise

Find D by iteration from Area of part full irregular section.

12.4 COMPUTATIONAL PROCEDURES

Assumptions are: Steady one dimentional flow and incompressible fluids.

12.4.1 BASIC EQUATIONS OF STEADY FLOW

a) Equation of Continuity

$A1.V1 = A2.V2 = Q$

b) Manning's Formula (friction slope)

$$Sf = \left\{ Q\,n/[1.486\,A\,(RH)^{\frac{2}{3}}] \right\}^2$$

c) Bernoulli's Equation (open flow)

$$D2 + HV2 + \Delta L\ Sfav = D1 + HV1 + \Delta L\ So \quad \text{where } HV = V^2/2g$$

d) Bernoulli's Equation (pressure flow)

$$D2 + HV2 + \Delta L\ Sfav + Hm = D1 + HV1 + \Delta L\ So$$

where Hm is miscel.losses.

e) Angle Point Loss

$$Hapt = 0.0033\,\Theta\,HV$$

Where Θ is deflection angle in degrees. The District recommends not to exceed 6°.

f) Bend Loss

$$HB = 0.2\,HV\,\sqrt{\Delta/90}$$

where Δ is central angle of bend in degrees.

g) Manhole Loss

Hmh = 0.05 HV (No. MN) where No. MH is number of manholes in a reach

h) Specific Energy

$$E = D + HV$$

i) Pressure - Momentum

$$P2 + M2 = P1 + M1 = F$$

where $M = (Q)^2 / (Ag)$

j) Critical Depth Dc

Dc is the depth of flow at minimun energy, to find Dc by parabolic method see References 12.6.4 otherwise iterate for Dc in the specific energy equat.

$$Ec = f(Dc) = Dc + HVC$$

k) Normal Depth Dn

Dn is the depth of uniform flow and is found by iteration from Manning's formula

$$A(RH)^{2/3} = f(Dn) = [Q\ n] / [1.486\ So^{1/2}]$$

12.4.2 REACH ANALYSIS

a) Open Flow

Intermediate points are computed on the W.S. profile in a reach using the standard step method. The difference in velocity head between two adjacent points is held to a maximum of ten per cent.

$$\Delta L = (E2 - E1) / (So - Sfav)$$

b) Pressure Flow

EGL 1 = EGL 2 + Hf + Hm

D1 = EGL 1 - HV1 - INV. 1

If W.S. profile rises to the soffit of a conduit before the end of the reach or if the H.G.L. breaks seal before the end of the reach, minor losses are adjusted to reflect only the portion of the reach under pressure.

Super Elevation (S.E.)

Super elevation is computed in curving channels as follows:

CHAN. TYPE 1: (Trap. Sect.)

Subcritical flow: $S.E. = 1.15 [HV/r] [b + D (ZL + ZR)]$

Supercritical flow: $S.E. = 2.6 [HV/r] [b + D (ZL + ZR)]$

CHAN. TYPE 2: (Rect. Sect.)

Subcritical flow: $S.E. = HV\ b/r$

Supercritical flow: $S.E. = 2\ HV\ b/r$

12.4.3 TRANSITION ANALYSIS

If V2 is greater than V1 then

$Ht = 0.1\ [HV2 - HV1]$

otherwise

$Ht = 0.2\ [HV1 - HV2]$

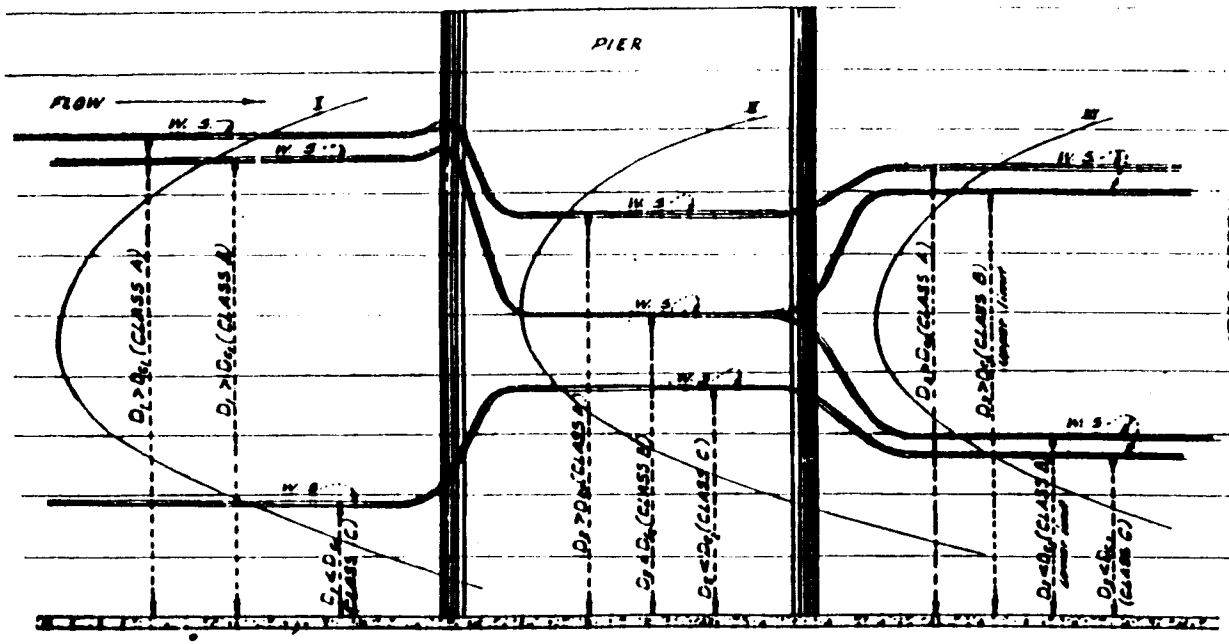
12.4.4 JUNCTION ANALYSIS

$\triangle Y = [(Q2.V2) - (Q1.V1) - (Q3.V3.COSO3)\ (1/g)\ (1/A\ ave)] + \triangle L\ Sf\ av$

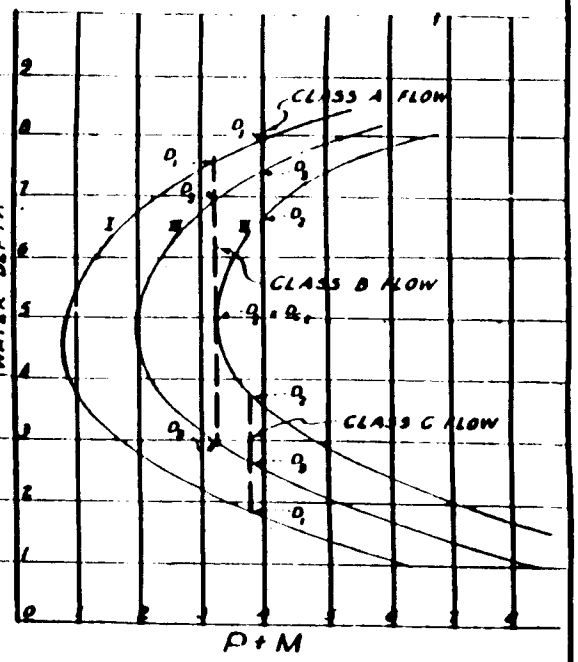
where A ave = $[(A1 + A2)/2]$

and $\triangle Y = D1 + \triangle H - D2$

$HJ = \triangle Y + HV1 - HV2$



LONGITUDINAL PROFILE



PRESSURE+MOMENTUM CURVES

GENERAL MOMENTUM EQUATION:

$$\underbrace{P_1 - P_p + \frac{Q^2}{gA_1}(A_1 - A_p)}_{I} = \underbrace{P_2 - P_p + \frac{Q^2}{gA_2}}_{II} = \underbrace{P_3 - P_p + \frac{Q^2}{gA_3}}_{III}$$

NOTATIONS

I, II, III = P+M curves upstream inside and downstream of bridge respectively

D_1, D_2, D_3 = Water depths upstream, inside and downstream of bridge respectively

D_{c_2} = Critical depth within bridge

W = Channel width.

$P_1 = P_3$ = Hydrostatic pressure in unobstructed channel.

P_p = Hydrostatic pressure of bridge piers.

$A_1 = A_3$ = Area of unobstructed channel in sq ft

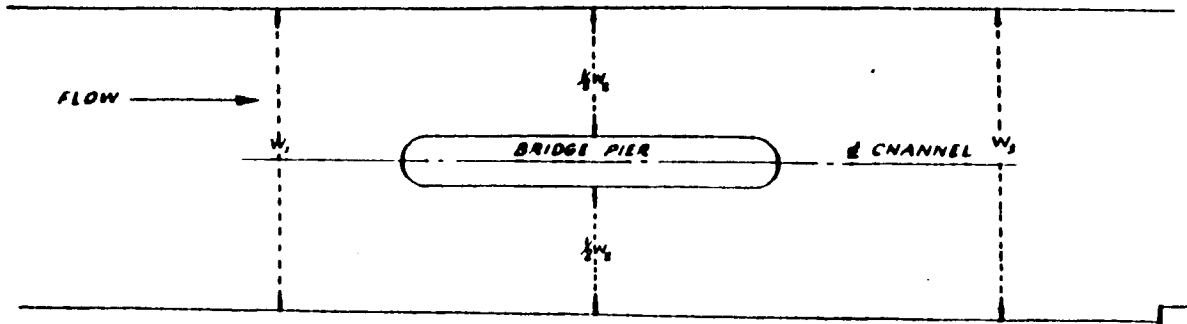
A_p = Area of bridge piers in sq ft

Q = Discharge in c f s

g = Gravitational constant

$D_{c_1} = D_{c_3}$ = Critical in unobstructed channel

* Use $\frac{1}{2} A_p$ for round nose piers



PLAN

Los Angeles County Flood Control District

BRIDGE PIER LOSSES BY THE MOMENTUM METHOD

12.4.6 WALL ENTRANCE ANALYSIS (Sudden Contraction)

Lower Stage Profile (U/S Control)

Find depth at the D/S end by iteration in the equation.

$$M2 + P2 = M1\ [(A1 - A1WALL) / A1] + P1 - P1\ walls$$

where A1 wall is the area of the obstructed part of A1. And P1 wall is the pressure on the obstructed part of A1

Upper Stage Profile (D/S Control)

If the control depth is less than the conduit height find the depth at the U/S end from

$$M2 + P2 = M1\ [(A1 - A1WALL) / A1] + P1 - P1\ wall$$

otherwise find D1 by iteration from the following equation:

$$D2 + HV2 + Kc\ ABS\ [HV2 - HV1] = D1 + HV1$$

where Kc ABS [HV2 - HV1] is the head loss at WE.

Kc = 0.5 unless given otherwise

ABS = the absolute value

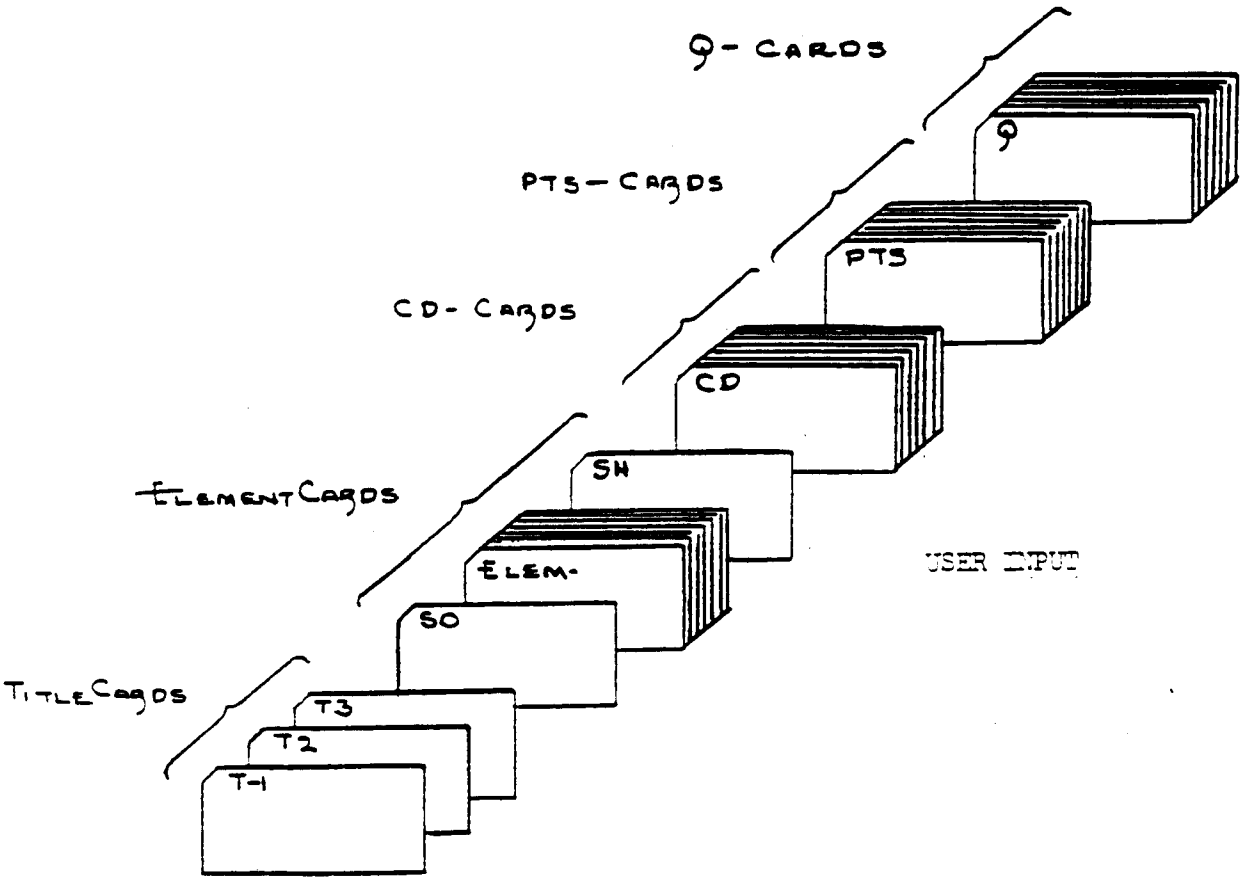
12.4.7 WALL EXIT (Sudden Expansion)

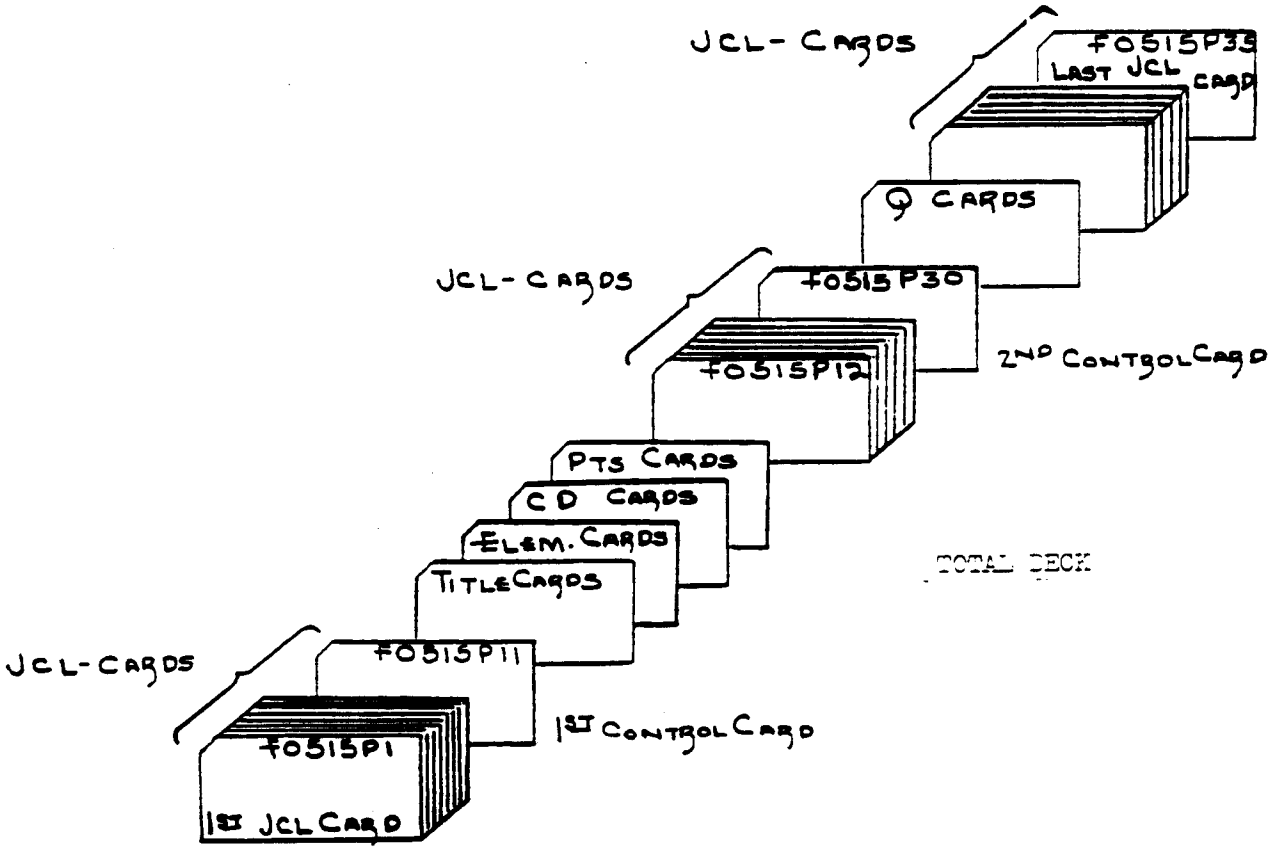
Energy loss in a wall exit = 1.0 ABS [HV2 - HV1]

In WX find D1 or D2 by iteration in the following

$$D2 + HV2 + 1.0\ ABS\ [HV2 - HV1] = D1 + HV1$$

12.5 PICTORIAL OF INPUT DECK





12.6 REFERENCES

1. Hydraulic Design Manual, L.A.C.F.C.D. (1970)

2. Handbook of Hydraulics, E. F. Brater and H. W. King

3. Hydraulic Analysis of Junctions, City of L.A. Office

   Std. No. 115 (1968)

4. Critical Water Surface by Minimum Specific Energy Using the Parabolic Method, Hydrologic Engineering Center, U.S. Army Corps of Engineers, Eichert, Bill S.